

TOWN OF RAVENEL REQUEST FOR PROPOSALS

RAVENEL HALL & BALL FIELD PARK IMPROVEMENTS

ADDENDUM 01

Addendum Date: May 19, 2023

This Addendum shall be considered part of the bid documents for the abovementioned project as thought it had been issued at the same time and shall be incorporated integrally therewith. Where provision of the following supplementary data differ from those of the original bid documents, this Addendum shall govern and take precedence.

BIDDERS MUST SIGN THE ADDENDUM AND SUBMIT IT WITH THEIR BIDS.

Proposers hereby notified that they shall make any necessary adjustments in their estimates as a result of this Addendum. It will be construed that each bidders's proposal is submitted with full knowledge of all modifications and supplemental data specified herein.

DUE DATE / TIME: The bidding due date and time have been extended. Proposals are now due: Friday, June 9, 2023 @ 2:00pm

Proposals received later then the 2:00pm shall be considered "LATE PROPOSALS."

BID OPENING will now be: Friday, June 9, 2023 @ 2:00pm

Except as below, the original bid document remains unchanged. The bid documents are modified and/or clarified as follows:

the following drawings:	iges allect
- G0.01	
- D1.01	
- A1.01	
- A1.03	
- A2.01	
- A3.01	
- A6.01	
BIDDERS MUST ACKNOWLEDGE THIS ADDENDUM BY SIGNING BEL	OW AND
ATTACHING THE SIGNED ADDENDUM TO THE BID FORM(s):	
ATTACHING THE SIGNED ADDENDUM TO THE BID FORM(s): Company Name:	-
Company Name:	

RAVENEL COMMUNITY HALL ADD & CONCESSIONS BLDG

BIDDING DOCUMENTS

5701 CONNERS STREET RAVENEL, SC 29470

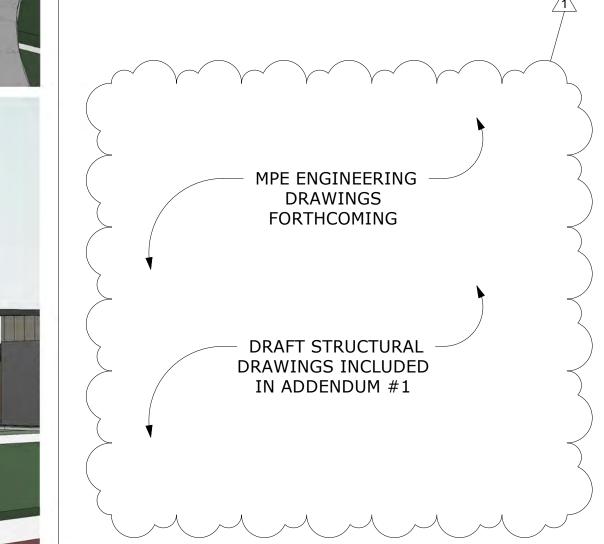
APRIL 19, 2023

CONTEXT MAP









SHEET INDEX

G0.02

G0.03

G0.04

D1.01

A1.01

A1.03

A1.10

A1.11

A2.01

A3.01

A3.02

A3.03

COVER SHEET

WALL TYPES

PROJECT INFORMATION

DEMOLITION PLAN - HALL

ROOF PLAN - HALL

CEILING PLAN - HALL

DEMOLITION ELEVATIONS - HALL

OVERALL SITE REFERENCE PLAN

FLOOR PLAN - HALL - NEW WORK

ROOF PLAN - CONCESSIONS

CEILING PLAN - CONCESSIONS

EXTERIOR ELEVATIONS - HALL

SECTION - STORAGE ADDITION

SECTION - SCREEN PORCH

SECTION - CONCESSIONS

FLOOR PLAN - CONCESSIONS - NEW WORK

EXTERIOR ELEVATIONS - CONCESSIONS

CODE ANALYSIS, LIFESAFETY FLOOR PLAN

SITE MAP



JOB TEAM

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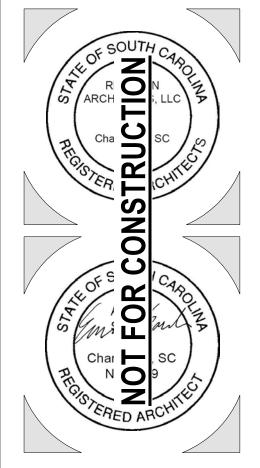
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RAVENEL CO DESCRIPTION ADDENDUM #1

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BIDDING DOCUMENTS

APPROVED FOR CONSTRUCTION

COVER SHEET

APRIL 19, 2023

G0.01

COMMUNITY HALL BUILDING PROJECT DESCRIPTION

THIS PROJECT IS A STORAGE ADDITION TO AN EXISTING HALL FOR THE USE OF THEIR CHAIRS AND TABLE SETS. THERE IS ALSO A SCREENED PORCH ADDITION TO THE REAR TO EXPAND THE USE OF THEIR EVENT SPACE. THERE IS NO CHANGE IN EXISTING EXITING/ EGRESS NUMBER OR BATHROOM FIXTURES. PATRONS OF THIS EVENT SPACE MAY ALSO UTILIZE THE NEW CONCESSIONS/BATHROOM BUILDING IF NEED ARISES FOR AN INCREASED NUMBER OF RESTROOM FIXTURES. BUILDING IS NOT EQUIPPED WITH A FIRE SUPPRESSION SYSTEM.

PLUMBING FIXTURE COUNT - IBC CHAPTER 29 -**BANQUET HALL***

1 FIXTURE PER 75 OCCUPANCY OF BUILDING, BASED ON PLUMBING AND SPRINKLER REQUIREMENTS, IS 300

*ALL PLUMBING FIXTURES ARE EXISTING TO REMAIN

BANQUET HALL 2127 SF NET SQUARE FEET 5 NET PER PERSON 355 PEOPLE STORAGE 228 SF KITCHEN 238 SF 705 SF PROPANE TANK D3 HALL - LIFESAFETY PLAN G0.02 1/8" = 1'-0"

CONCESSIONS BUILDING PROJECT DESCRIPTION

THIS PROJECT IS A NEW CMU BLOCK MASONRY AND WOOD TRUSS BUILDING TO HOUSE STORAGE, CONCESSIONS, AND RESTROOM FACILITIES FOR THE BALLFIELD AND SURROUNDING PARK AREAS. IT IS TYPE V COMBUSTIBLE CONSTRUCTION. THE STORAGE WILL BE FOR NON HAZARDOUS BASEBALL EQUIPMENT. SOME CONCESSION STORAGE WILL ALSO BE INCLUDED. BUILDING IS NOT EQUIPPED WITH A FIRE SUPPRESSION SYSTEM.

PLUMBING FIXTURE COUNT - IBC CHAPTER 29 -**OUTDOOR SPORTING EVENTS**

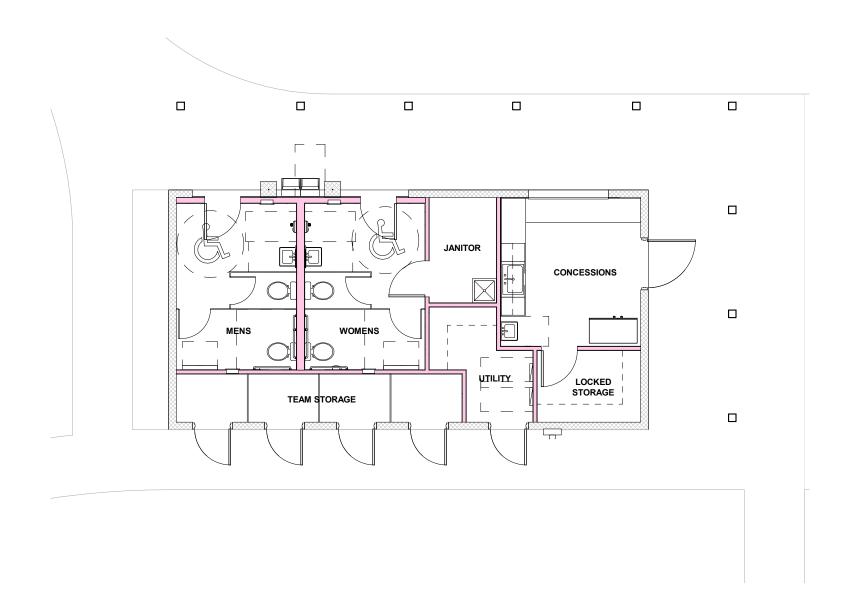
1 FIXTURE PER 75 MALE (FOR THE FIRST 1,500) 1 FIXTURE PER 40 FEMALE (FOR THE FIRST 1,520) OCCUPANCY OF BALLFIELD, BASED ON PLUMBING, IS 230

TOTAL TOILETS REQUIRED 4 PROVIDED 4

DRINKING FOUNTAINS REQUIRED 2

TOTAL SINKS PROVIDED 3

PROVIDED 2 **SERVICE SINK REQUIRED 1** PROVIDED 1



CONCESSIONS - LIFESAFETY

GENERAL CODE INFORMATION

APPLICABLE BUILDING CODES

2021 SOUTH CAROLINA BUILDING CODE 2021 SOUTH CAROLINA FIRE CODE 2021 SOUTH CAROLINA PLUMBING CODE 2021 SOUTH CAROLINA MECHANICAL CODE 2021 SOUTH CAROLINA FUEL GAS CODE 2009 INTERNATIONAL ENERGY CONSERVATION CODE 2017 NATIONAL ELECTRICAL CODE

LIFESAFETY LEGEND

1 HOUR RATED WALL 2 HOUR RATED WALL 3 HOUR RATED WALL EXIT ACCESS TRAVEL DISTANCE EXIT SIGN FEC - FIRE EXTINGUISHER CABINET

HALL BUILDING - PARTIAL CODE REQUIREMENTS**

<u>ITEM</u>	CODE SECTION	DESCRIPTION
OCCUPANCY:	303	ASSEMBLY, GROUP A3 - COMMUNITY HALL
BUILDING HEIGHT IN FEET:	TABLE 504.3 GROUP A:	ALLOWABLE: 40'-0" ACTUAL: 18'-0"
BUILDING STORIES:	TABLE 504.4 GROUP A3:	ALLOWABLE: ONE STORY ACTUAL: ONE STORY
BUILDING AREA:	TABLE 506.2 GROUP A3:	ALLOWABLE: 6,000 GSF EXISTING: 3,000 GSF ADDITION: 1,018 GSF
		TOTAL: 4,018 GSF
NON-SEPARATED OCCUPANCY	508.3.2	MOST RESTRICTIVE APPLIES TO ENTIRE BUILDING
SEPARATION REQUIREMENTS	TABLE 508.4	NONE REQUIRED
FIRE-RESISTANCE RATING REQUIREMENTS	TABLE 601	NONE REQUIRED
CONSTRUCTION TYPE:	602.2	TYPE VB
SPRINKLER NOTES:	903.2	EXISTING BUILDING NOT SPRINKLERED, BUT NOT REQUIRED - MAX OCCUPANCY IS 300
OCCUPANT LOAD	TABLE 1004.1.1	ASSEMBLY AREAS - MAX 300 PER BELOW
	TOTAL	300 MAX TOTAL OCCUPANTS FOR PLUMBING & SPRINKLER REQUIREMENTS
EXIT ACCESS TRAVEL DISTANCE:	TABLE 1017.2	200'-0" ALLOWABLE
ACCESSIBILITY:		PROVIDED ACCORDING TO IBC CHAPTER 11 REQUIREMENTS AND ANSI A117 2017

CONCESSIONS RIDG - DARTIAL CODE REQUIREMENTS**

<u>ITEM</u>	CODE SECTION	DESCRIPTION
OCCUPANCY:	304 311	BUSINESS, GROUP B - CONCESSIONS & RESTROOMS STORAGE, GROUP S1 - LEATHER & FOOD PRODUCTS*
BUILDING HEIGHT IN FEET:	TABLE 504.3 GROUP B/S:	ALLOWABLE: 40'-0" ACTUAL: 16'-0"
BUILDING STORIES:	TABLE 504.4 GROUP S1*:	ALLOWABLE: ONE STORY ACTUAL: ONE STORY
BUILDING AREA:	TABLE 506.2 GROUP B/S1*:	ALLOWABLE: 9,000 GSF ACTUAL: 800 GSF
NON-SEPARATED OCCUPANCY	508.3.2	MOST RESTRICTIVE APPLIES TO ENTIRE BUILDING *MOST RESTRICTIVE IS S1
SEPARATION REQUIREMENTS	TABLE 508.4	NONE REQUIRED
FIRE-RESISTANCE RATING REQUIREMENTS	TABLE 601	NONE REQUIRED
CONSTRUCTION TYPE:	602.5	TYPE VB
SPRINKLER NOTES:	903.2	NOT REQUIRED
OCCUPANT LOAD	TABLE 1004.1.1	800/150 = 6 PEOPLE
EXIT ACCESS TRAVEL DISTANCE:	TABLE 1017.2	200'-0" ALLOWABLE
ACCESSIBILITY:		PROVIDED ACCORDING TO IBC CHAPTER 11 REQUIREMENTS AI ANSI A117 2017

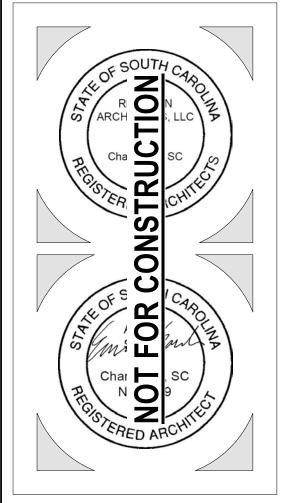
** THE ABOVE MATRICES IS NOT INTENDED TO ENCOMPASS ALL THE CODES AND RESTRICTIONS SET FORTH BY THE INTERNATIONAL BUILDING CODE, 2021 EDITION. IT IS INTENDED TO LIST THE APPLICABLE PERTINENT INFORMATION, AND DOES NOT OVERRIDE LIMITATIONS SET FORTH BY ANY CHAPTERS OR SECTIONS OF THE IBC 2021.

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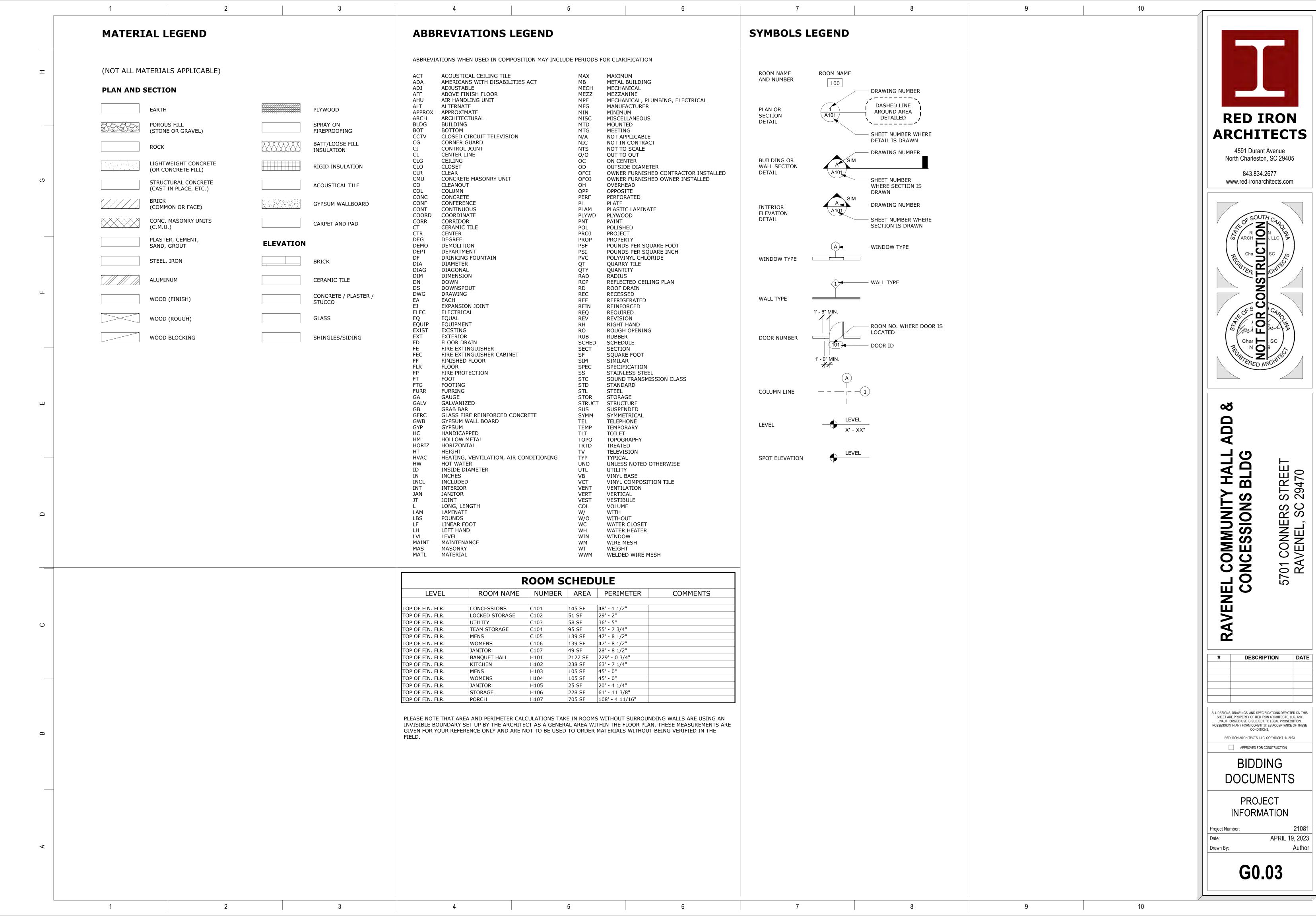
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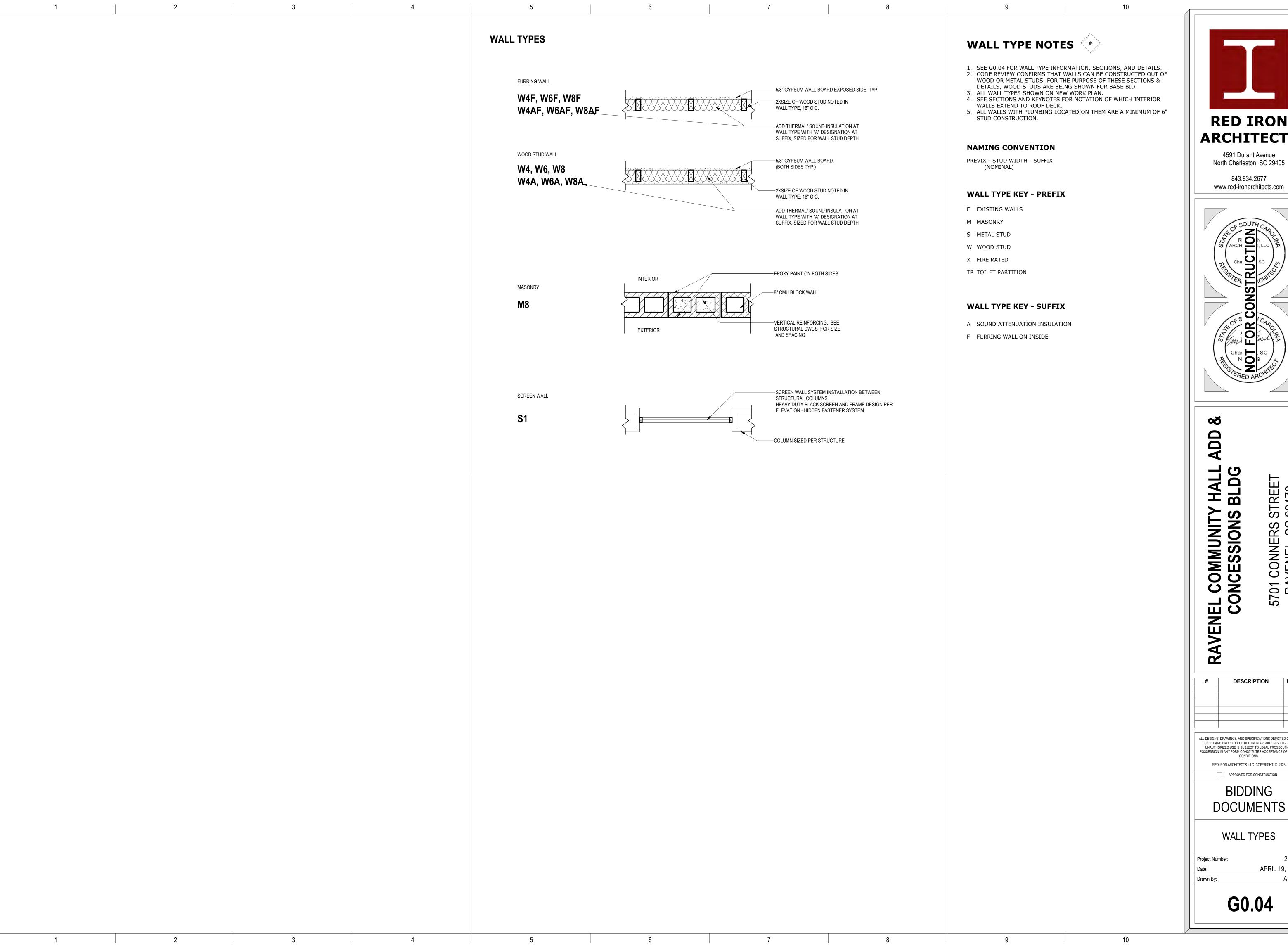
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LIFESAFETY FLOOR Project Number:

APRIL 19, 2023 Drawn By:

G0.02

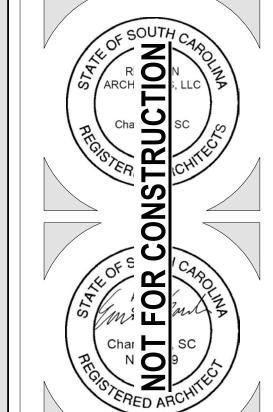




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BIDDING DOCUMENTS

WALL TYPES

21081 APRIL 19, 2023

DIVISION 01.1 - GENERA STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL ELECTRICAL, SHOP DRAWINGS AND SPECIFICATIONS. 2. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA, AND TO SUBMIT DOCUMENTS TO ALL SUBCONTRACTORS AND SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS THE GENERAL CONTRACTOR SHALL COMPARE ALL CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN DISCIPLINES OR WITHIN A GIVEN DISCIPLINE TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND ERECTION 4. IF A CONFLICT EXISTS WITHIN THE STRUCTURAL DRAWINGS, GENERAL NOTES, OR THE SPECIFICATIONS, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE STRUCTURAL ENGINEER, SHALL GOVERN. THE CONTRACTOR SHALL COORDINATE ALL ELEVATIONS AND DIMENSIONS, INCLUDING BUT NOT LIMITED TO THOSE FOR OPENINGS IN WALLS, ROOF(S), AND FLOOR SYSTEM(S), WITH THE ARCHITECTURAL, PLUMBING, ELECTRICAL, AND MECHANICAL PLANS. ALL DIMENSIONS, ELEVATIONS, AND ANY OTHER CONDITIONS OF ANY EXISTING STRUCTURES OR OTHER FEATURES SHALL BE VERIFIED BY THE GENERAL CONTRACTOR AND ANY DISCREPANCIES WITH THE CONTRACT DRAWINGS REPORTED TO THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DURING THE CONSTRUCTION PROCESS, IT SHALL BE SOLELY THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN THE INTEGRITY OF THE EXISTING STRUCTURE AND TO PROTECT FROM DAMAGE ANY PORTIONS THAT ARE TO REMAIN.

THE COMPLETED LATERAL-FORCE RESISTING SYSTEMS AND DIAPHRAGMS ARE REQUIRED FOR THE STRUCTURE TO RESIST LATERAL LOADS AND PROVIDE STABILITY UNDER GRAVITY LOADS. DURING THE CONSTRUCTION PROCESS, THE CONTRACTOR SHALL PROVIDE ALL REQUIRED BRACING TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.

UNLESS OTHERWISE NOTED, DETAILS SHOWN ON ANY DRAWING ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.

9. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS AND METHODS AND FOR SAFETY PRECAUTIONS AND PROGRAMS

10. THE ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE ACTS OR OMISSION OF THE CONTRACTOR OR FOR THEIR FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE BUILDING OWNER SHALL PROVIDE PERIODIC MAINTENANCE TO INSURE STRUCTURAL INTEGRITY. SUCH MAINTENANCE SHALL INCLUDE BUT NOT LIMITED TO PAINTING OF STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS.

DIVISION 01.2 - DESIGN CRITERIA (32.762820, LONG: -80.234316)

THE CONTRACT DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE 2021 INTERNATIONAL BUILDING CODE

 2021 INTERNATIONAL EXISTING BUILDING CODE (LEVEL 2 ALTERATION) ASCE 7-16 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"

DEAD LOADS: a. TYPICAL ROOF SYSTEMS (PSF): STANDING SEAM METAL DECK: **BUILDING WRAP:** INSULATION: 3.0 (ALLOWANCE) 5/8 WOOD SHEATHING: CEILING: 3.0 (ALLOWANCE)

WD FRAMING: 5.0 (ALLOWANCE) TOTAL LOAD:

(*) INCLUDES MISC. CEILING AND HANGING MECHANICAL LOADS SUCH AS DUCT WORK AND LIGHTS.

a. LIVE LOADS ARE BASED ON THE MORE RESTRICTIVE OF THE UNIFORM LOAD LISTED BELOW OR THE CONCENTRATED LOAD LISTED ACTING OVER A 2.5-FOOT BY 2.5-FOOT AREA (OR 6.25 SF). b. CATEGORY CONCENTRATED UNIFORM

20 PSF ORDINARY ROOF SNOW LOADS:

5 PSF GROUND SNOW LOAD, PG WIND LOADS: ULTIMATE WIND SPEED, V(ult) 142 MPH (3 SEC GUST) NOMINAL WIND SPEED, V(asd) 110 MPH RISK CATEGORY WIND EXPOSURE

INTERNAL PRESSURE COEFF, GCPI ±0.18 (ENCLOSED) COMPONENTS AND CLADDING WIND LOADS AT WAREHOUSE (ULTIMATE, Kd = 0.85): (10 SQ FT EFF WIND AREA) (50 SQ FT EFF WIND AREA) = -40 PSF/ +30 PSF = -35 PSF/ +25 PSF EXT ZONE (10 SQ FT EFF WIND AREA) (50 SQ FT EFF WIND AREA) ZONE 1 = -40 PSF/ +16 PSF = -50 PSF/ +16 PSF ZONE 2 = -65 PSF/ +16 PSF = -55 PSF/ +16 PSF ZONE 3 = -85 PSF/ +16 PSF = -70 PSF/ +16 PSF (10 SQ FT EFF WIND AREA) (50 SQ FT EFF WIND AREA) ZONE 1 = -50 PSF = -45 PSF ZONE 2 = -65 PSF = -50 PSF

= -65 PSF

E-L-F

X = 3.0in; Y = PER EXIST

SEISMIC LOADS: RISK CATEGORY

SEISMIC IMPORTANCE FACTOR, le

ZONE 3

SHORT PERIOD SPECTRAL RESPONSE ACCELERATION, S, 1.667g 1-SEC PERIOD SPECTRAL RESPONSE ACCELERATION, S1 0.514g SITE CLASS D (DEFAULT) SHORT PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION, SDS. 1-SEC PERIOD DESIGN SPECTRAL RESPONSE ACCELERATION, S_{D1} 0.617g SEISMIC DESIGN CATEGORY CONCESSIONS BASIC SEISMIC-FORCE RESISTING SYSTEM: SPECIAL REINFORCED MASONRY SHEAR WALLS **DESIGN BASE SHEAR:** 0.267 SEISMIC RESPONSE COEFFICIENT, Cs: 5.0 RESPONSE MODIFICATION COEFFICIENT, R: ANALYSIS PROCEDURE: **SIMPLIFIED** BASIC SEISMIC-FORCE RESISTING SYSTEM: CANTILEVERED TIMBER FRAMES **DESIGN BASE SHEAR:** 13.4 KIPS SEISMIC RESPONSE COEFFICIENT, Cs: 0.889 RESPONSE MODIFICATION COEFFICIENT, R: 1.5

= -85 PSF

EDGE & CORNER ZONE, 0.6h = 9.0 FT; 0.2h = 3.0 FT, a = 3.0 FT

CONTACT EOR FOR C&C LOADS AT OTHER EFFECTIVE WIND AREAS

THE GENERAL CONTRACTORS SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR REVIEW. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND/OR ENGINEER AND HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED PRIOR TO FABRICATION. FABRICATION AND ERECTION SHALL BE FROM REVIEWED SHOP DRAWINGS.

A RECORD SET OF APPROVED SHOP DRAWINGS SHALL BE KEPT IN THE FIELD BY THE GENERAL CONTRACTOR. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY

PART OF THE STRUCTURE DETAILED ON THE CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED

THE CONTRACTOR SHALL PREPARE A LIST AND SCHEDULE OF ALL STRUCTURAL SUBMITTALS PRIOR TO CONSTRUCTION.

5. THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR FOR THE ENGINEER'S REVIEW: ROOF TRUSSES (1,3)

STRUCTURAL STEEL STRUCTURAL STEEL TO STEEL CONNECTIONS (1,3)

REINFORCING STEEL

CONCRETE MIX DESIGNS

MAY OCCUR HEREON.

ANALYSIS PROCEDURE

AMPLIFIED DRIFT:

ITEMS MARKED (1) SHALL HAVE SHOP DRAWINGS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS LOCATED. ITEMS MARKED (2) SHALL BE SUBMITTED TO ENGINEER FOR OWNER'S RECORD ONLY AND WILL NOT HAVE THE ENGINEER'S SHOP DRAWING STAMP AFFIXED. ITEMS MARKED (3) SHALL HAVE DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE STATE WHERE THE PROJECT IS

CONTRACTOR SHALL SUBMIT ONE SET OF REPRODUCIBLES AND TWO SETS OF PRINTS FOR ALL SHOP DRAWINGS SPECIFIED TO BE RETURNED BY THE ENGINEER.

THE OMISSION FROM THE SHOP DRAWINGS OF ANY MATERIALS REQUIRED BY THE CONTRACT DOCUMENTS TO BE FURNISHED SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY OF FURNISHING AND INSTALLING SUCH MATERIALS, REGARDLESS OF WHETHER THE SHOP DRAWINGS

HAVE BEEN REVIEWED AND APPROVED. THE USE OF ELECTRONIC FILES OR REPRODUCTIONS OF THESE CONTRACT DOCUMENTS BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES THEIR ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATES THEMSELVES TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT

DIVISION 03.1 - CONCRETE

1. CONCRETE SHALL HAVE THE MINIMUM STRENGTH AND MEET THE PROPERTIES AS DESCRIBED BELOW FOR THE VARIOUS CLASSES OF CONCRETE: MIX TYPE SUPER P SLUMP | W/C RATIO | DURABILITY LOCATION F0, S0, W0, C1 3000 PSI N-A 4" .50 MAX FOOTINGS 4000 PSI F1, S1, W1, C1 SLABS & PIERS REQUIRED 3" / 8" .45 MAX

a. ALL CONCRTE SHALL BE NORMAL WEIGHT CONCRETE 2. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE".

3. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE II. 4. ALL AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C 33.

5. ALL REINFORCEMENT SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS: a. ALL REINFORCING, UNO: ASTM A615 GRADE 60

ASTM A496 (75 KSI) b. DEFORMED BAR ANCHORS (DBA): WELDED REINFORCING: ASTM A706

d. WELDED WIRE REINFORCEMENT (WWR) SMOOTH WIRE: ASTM A 185 (65 KSI)

 DEFORMED WIRE ASTM A 497 (70 KSI) REINFORCEMENT DETAILING:

ELEMENTS, UNLESS OTHERWISE INDICATED

a. REINFORCEMENT SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI 315. b. PROVIDE CORNER BARS AT ALL FOOTINGS AND WALL INTERSECTIONS TO MATCH HORIZONTAL REINFORCING SIZE AND SPACING. AT INTERSECTIONS OF CONTINUOUS SPREAD FOOTINGS EXTEND ALL

BARS TO FAR SIDE OF INTERSECTING FOOTING c. REINFORCEMENT SHALL BE SECURELY PLACED TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT. PROVIDE CONCRETE COVER FOR REINFORCING PER ACI 318, UNLESS SPECIFICALLY

d. PROVIDE DOWELS TO MATCH REINFORCEMENT SIZE AND SPACING INDICATED FOR ALL STRUCTURAL

LAP BARS AT ALL SPLICES, INCLUDING CORNER BARS AND DOWELS, IN ACCORDANCE WITH SPLICE SCHEDULE OR, IN LIEU THEREOF, 40 BAR DIAMETERS. LAP WWF 6" OR ONE FULL MESH, WHICHEVER IS GREATER.

8. CONCRETE PROTECTION FOR REINFORCING: 3" AT FOOTINGS; 2" AT FORMED SURFACES LATER EXPOSED TO

9. HORIZONTAL CONSTRUCTION JOINTS IN CONCRETE POURS SHALL NOT BE USED UNLESS SHOWN ON THE DRAWINGS. THE ARCHITECT/ENGINEER SHALL APPROVE ALL DEVIATIONS OR ADDITIONAL JOINTS IN WRITING. 10. SLABS AND BEAMS OR JOISTS SHALL BE CAST MONOLITHICALLY UNLESS OTHERWISE INDICATED. 11. CHAMFER ALL PERMANENTLY EXPOSED CONCRETE EDGES 3/4 INCH, UNO.

12. NO HOLES OR OPENINGS THROUGH FOUNDATION WALLS AND/OR FOOTINGS WITHOUT ENGINEER'S

13. AIR ENTRAIN ALL EXTERIOR CONCRETE AS REQUIRED BY ACI 318-19 TABLE 19.3.3.1. 14. ALUMINUM SHALL NOT BE EMBEDDED IN ANY CONCRETE.

HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL CONFORM TO ASTM C90, LIGHTWEIGHT, TYPE II, WITH A

MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI ON THE NET AREA (fm = 2000 PSI). 2. ALL MORTAR FOR USE IN MASONRY SHALL CONFORM TO ASTM C270, TYPE M OR S. ALL GROUT FOR USE IN MASONRY SHALL CONFORM TO ASTM C476, MIN 2500 PSI. GROUT SLUMP SHALL BE 8"-11". REINFORCING BARS SHALL MEET ASTM A615, GRADE 60.

BOND BEAM BLOCKS SHALL MEAN OPEN BOTTOM LINTELS UNLESS NOTED OTHERWISE. PROVIDE METAL LATH IN JOINTS BELOW BOTTOM OF BOND BEAM OVER THOSE CELLS THAT ARE NOT GROUT FILLED. VERTICAL AND HORIZONTAL REINFORCING SHALL BE CONTINUOUS AND LAPPED PER IBC 2107.2.1 (EQ 21-1)

BUT NOT BE LESS THAN 12" OR 40db BUT NEED NOT BE GREATER THAN 72db. 6. HOLD VERTICAL BARS STRAIGHT, TRUE, AND ACCURATE IN ALL WALLS AS DETAILED. INSTALL REBAR POSITIONERS @ 4'-0" OC, MAXIMUM, THAT ARE DESIGNED TO HOLD REBAR IN PROPER LOCATION WITHIN THE

CELL PRIOR TO GROUTING. 7. PROVIDE A MINIMUM OF 1/2" GROUT BETWEEN MAIN REINFORCING AND MASONRY UNITS PROVIDE STANDARD GAGE TRUSS TYPE JOINT REINFORCEMENT AT 16" OC FOR TYPICAL HORIZONTAL

REINFORCING, UNLESS NOTED OTHERWISE. 9. ALL REINFORCED MASONRY COLUMN AND WALL SECTIONS REQUIRE DOWELS FROM FOOTING, SAME SIZE AND QUANTITY AS VERTICAL REINFORCEMENT IN COLUMN OR WALL UNLESS NOTED OTHERWISE.

10. GROUT FILL ALL CELLS, ALL WALLS BELOW GRADE. SLUSH JOINT BETWEEN WYTHES BELOW GRADE. ALL CMU SHALL BE LAID IN RUNNING BOND PATTERN.

12. GROUT FILL ALL CELLS THAT CONTAIN REINFORCING.

13. CONSTRUCTION OF ALL MASONRY SHALL CONFORM TO THE SPECIFICATION FOR MASONRY STRUCTURES (TMS 602-16). INCLUDED ARE CONSTRUCTION TOLERANCES; THE PLACEMENT, BONDING, AND ANCHORING OF MASONRY; AND THE PLACEMENT OF GROUT AND OF REINFORCEMENT 14. THE TOP OF EACH GROUT POUR SHALL BE 1" BELOW THE BED JOINT

15. REINFORCEMENT, REBAR POSITIONERS, AND TIES SHALL BE PLACED PRIOR TO GROUTING.

UNLESS NOTED OTHERWISE, ALL LUMBER SHALL BE #2 SOUTHERN YELLOW PINE WITH A MAXIMUM MOISTURE CONTENT OF 19% (OR BETTER).

2. ALL LUMBER EXPOSED TO THE EXTERIOR ENVIRONMENT SHALL BE PRESSURE IMPREGNATED AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK "ABOVE GROUND USE". 3. ALL LUMBER IN CONTACT WITH THE GROUND/CONCRETE SHALL BE PRESSURE IMPREGNATED AND EACH

PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK "GROUND CONTACT USE." 4. WHERE PRESERVATIVE TREATED LUMBER IS CUT OR DRILLED AFTER TREATMENT, THE CUT SURFACE AND

DRILLED HOLES SHALL BE FIELD TREATED IN ACCORDANCE WITH AWPA M4. 5. ALL LUMBER DESIGNATED AS "LVL" SHALL BE MICROLLAM LAMINATED VENEER LUMBER WITH E=2,000,000 PSI

AND Fb = 2600 PSI AT 100% LOAD DURATION (ICC-ES REPORT ESR-1387). AT PORCH ROOFS PROVIDE CROSS BRIDGING OR BLOCKING AT 4'-0" OC (MAX).

1. WALL FRAMING SHALL BE SPRUCE-PINE-FIR #1/#2 OR BETTER (UNO). 2. ALL BEAMS AND JOISTS SHALL BE SEAT CUT FOR FULL UNIFORM BEARING AT SUPPORTS, BEAM SEATS AND

COLUMN CAPS. 3. ALL WOOD IN CONTACT WITH CONCRETE OR GRADE OR WITHIN THE DFE SHALL BE PRESERVATIVE TREATED. 4. ALL BOLTED OR NAILED STRAP CONNECTIONS SHALL HAVE AN EQUAL NUMBER OF BOLTS OR NAILS EACH SIDE OF THE SPLICE JOINT. THE FIRST BOLT OR NAIL FROM EACH SIDE OF THE SPLICED OR TREATED MEMBER SHALL BE EQUAL DISTANCE FROM THE SPLICE. STRAPS USING 16d NAILS ON 2x MATERIAL SHALL BE

INSTALLED ON THE 1-1/2" EDGE OF THE MEMBER. 5. THE CONTRACTOR SHALL VERIFY THAT THE MOISTURE CONTENT OF ALL FRAMING LUMBER AND PLYWOOD MEET THE REQUIREMENTS OF THE SPECS. AT THE TIME OF INSTALLATION AND AT CLOSE-IN.

PROVIDE DOUBLE STUDS UNDER DOUBLE TRUSSES/ RAFTERS. PROVIDE TRIPLE STUDS UNDER TRIPLE TRUSSES/ RAFTERS. FIELD LOCATE ROOF AND FLOOR FRAMING FOR LOCATIONS.

DIVISION 06.3 - WOOD FASTENERS

 AS A MINIMUM, ANCHOR AND NAIL FRAMING SHALL COMPLY WITH THE FASTENING SCHEDULE ON THIS SHEET. ALL NAILS SHALL BE COMMON WIRE NAILS UNLESS NOTED OTHERWISE. 3. FASTENERS, INCLUDING NUTS AND WASHERS, IN CONTACT WITH PRESSURE TREATED OR FIRE RETARDANT

TREATED WOOD SHALL BE OF TYPE 304 OR 316 STAINLESS STEEL (UNLESS RECOMMENDED OTHERWISE BY THE TREATMENT MANUFACTURER). STAINLESS STEEL PARTS AND GALVANIZED STEEL PARTS SHALL NOT BE PLACED IN CONTACT WITH ONE ANOTHER. 4. ALL STEEL BOLTS SHALL MEET ASTM A307 (UNO). BOLT HOLES SHALL BE A MAXIMUM 1/16" LARGER THAN THE

BOLT DIAMETER AND SHALL BE PRE-DRILLED PRIOR TO DELIVERY. BOLTS SHALL NOT BE FORCIBLY DRIVEN. 5. ALL LAG SCREWS SHALL MEET ASTM A307 (UNO). THE CLEARANCE HOLE FOR THE SHANK SHALL HAVE THE SAME DIAMETER AS THE SHANK, AND THE SAME DEPTH OF PENETRATION AS THE LENGTH OF UNTHREADED SHANK. THE LEAD HOLE FOR THE THREADED PORTION SHALL HAVE A DIAMETER EQUAL TO 60% TO 75% OF THE SHANK DIAMETER AND A LENGTH EQUAL TO AT LEAST THE LENGTH OF THE THREADED PORTION. THE THREADED PORTION OF THE LAG SCREW SHALL BE INSERTED IN ITS HOLE BY TURNING WITH A WRENCH, NOT

BY DRIVING WITH A HAMMER. SOAP OR OTHER LUBRICANT SHALL BE USED ON THE LAG SCREWS OR IN THE

LEAD HOLES TO FACILITATE INSERTION AND PREVENT DAMAGE TO THE LAG SCREW. ALL NUTS SHALL MEET ASTM A563 (UNO).

2-3/4" INTO FRAMING (UNO).

ALL WASHERS SHALL MEET ASTM F844 (UNO). METAL CONNECTORS IN CONTACT WITH PRESSURE TREATED WOOD SHALL HAVE A MINIMUM FINISH/COATING MEETING ASTM A653 TYPE G185 ZINC-COATED GALVANIZED STEEL OR POST HOT-DIPPED GALVANIZED PER ASTM A123 PROVIDING A MINIMUM AVERAGE COATING WEIGHT OF 2.0 OUNCES PER SQ FT (TOTAL FOR BOTH

SIDES).

9. EDGE DISTANCES, END DISTANCES, AND FASTENER SPACINGS SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

10. INSTALL ALL PROPRIETARY PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. 11. CONNECTORS INDICATED IN DRAWINGS ARE SIMPSON STRONG-TIE PRODUCTS (UNO). a. SIMPSON STRONG-TIE CONNECTORS ARE SPECIFICALLY REQUIRED TO MEET THE STRUCTURAL

UPON RELIABLE PUBLISHED DATA OR CALCULATIONS. THE ENGINEER OF RECORD (EOR) SHALL EVALUATE AND GIVE WRITTEN APPROVAL FOR SUBSTITUTION PRIOR TO INSTALLATION. USE SIMPSON STRONG-TIE FASTENERS (NAILS, SCREWS) PER MANUFACTURER'S RECOMMENDATIONS.

CALCULATIONS OF PLAN. BEFORE SUBSTITUTING ANOTHER BRAND, CONFIRM LOAD CAPACITY BASED

ALL SIMPSON STRONG-TIE SDS WOOD SCREWS SHALL HAVE 3/4"Ø WASHERS AND A MIN EMBEDMENT OF

DIVISION 06.4 - PRE-ENGINEERED WOOD TRUSSES STRUCTURAL DRAWINGS 1. ALL PREFABRICATED TRUSSES SHALL BE DESIGNED TO MEET THE LOADING SPECIFIED. FABRICATION AND ERECTION SHALL BE PER TRUSS PLATE INSTITUTE RECOMMENDATIONS AS CONTAINED IN THE APPROPRIATE S002 QUALITY ASSURANCE PLAN

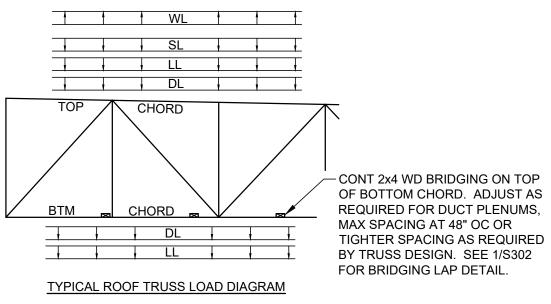
SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR EACH TYPE OF TRUSS SPECIFIED AND SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF SOUTH CAROLINA.

3. ALL LUMBER USED IN THE MANUFACTURING OF WOOD TRUSSES SHALL BE SOUTHERN YELLOW PINE OR

PROVIDE ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING AS REQUIRED AND SHOWN ON THE TRUSS MANUFACTURERS SHOP DRAWINGS. BRACING SHOWN ON STRUCTURAL DRAWINGS IS IN ADDITION TO ANY BRACING SHOWN ON THE SHOP DRAWINGS.

PROVIDE 2x4 CONTINUOUS BOTTOM CHORD/ CEILING BRACING AS INDICATED.

6. INSTALL STRONG BACKS, BRACING AND/OR BRIDGING PRIOR TO DECK INSTALLATION AND AS TRUSSES ARE ERECTED. FASTEN ALL BRACING TO EACH TRUSS.



THIS DIAGRAM IS A SCHEMATIC SHOWING THE APPLICATION OF LOADS STATED HEREIN. LOADS SHALL BE COMBINED AS PROVIDED BY THE GOVERNING BUILDING CODE TO DETERMINE THE MOST UNFAVORABLE EFFECT.

DL = 10 PSF BOTTOM CHORD DL = 10 PSF TOP CHORD LL = 20 PSF TOP CHORD LL = 0 PSF BOTTOM CHORD

ALL ROOF AND WALL SHEATHING SHALL BE INSTALLED WITH LONG DIMENSION PERPENDICULAR TO FRAMING AND END JOINTS SHALL BE STAGGERED. DESIGN IS BASED ON 2-SPAN CONTINUOUS CONDITION.

SNOW, Pg = 5 PSF

2. DO NOT USE PANELS LESS THAN 12".

WIND LOAD = 142 MPH

3.1. ROOF DECKING SHALL BE APA RATED 40/20 - 5/8" EXPOSURE 1 PLYWOOD PANELS ATTACHED WITH 8d ANNULAR RING SHANK NAILS (D=0.131, L=2-1/2, TL=1-1/2, H=0.281) AT 6" OC AT SUPPORTED EDGES, NAIL SPACING AT DIAPHRAGM BOUNDARY (EDGE NAILING) IS 6" OC, AND 6" OC IN FIELD. AT PORCHES WITH RAFTER SPACINGS OF 16" OC OR LESS, ATTACH SUPPORTED EDGES AND FIELD OF PANEL TO FRAMING WITH NAILS AT 6" OC. SUPPORT ALL PANEL EDGES PER DET 2/S302. LRFD WIND LOAD CAPACITY = 505 PSF (SDPWS TABLE 3.2.2 - STRENGTH AXIS PERPENDICULAR TO SUPPORTS).

3.2. EXTERIOR WALL SHEATHING SHALL BE APA RATED 24/16 - 7/16" EXPOSURE 1 PLYWOOD PANELS ATTACHED WITH 8d ANNULAR RING SHANK NAILS (D=0.131, L=2-1/2, TL=1-1/2, H=0.281) AT 6" OC AT SUPPORTED EDGES, NAIL SPACING AT DIAPHRAGM BOUNDARY IS 6" OC, AND 6" OC IN FIELD (UNO). LRFD WIND LOAD CAPACITY = 260 PSF (SDPWS TABLE 3.2.1A - STRENGTH AXIS PERPENDICULAR TO

3.3. WHERE NOTED ON PLAN, FLOOR SHEATHING SHALL BE SINGLE-LAYER STRAIGHT LUMBER. LUMBER SHALL BE MINIMUM 1" THICK NOMINAL SHEATHING BOARDS OR MINIMUM 2" THICK NOMINAL LUMBER LAID PERPENDICULAR TO SUPPORTS. END JOINTS IN ADJACENT BOARDS SHALL BE SEPARATED BY AT LEAST ONE JOIST SPACE AND THERE SHALL BE AT LEAST TWO BOARDS BETWEEN JOINTS ON THE SAME SUPPORT. ATTACH 1x LUMBER TO INTERMEDIATE AND END SUPPORTS USING A MINIMUM OF (3) 8d COMMON NAILS. ATTACH 2x LUMBER TO INTERMEDIATE AND END SUPPORTS USING A MINIMUM OF (3) 16d COMMON NAILS.

4. OPENINGS IN THE FLOOR OR ROOF DECK WITH A DIMENSION PERPENDICULAR TO THE JOISTS GREATER THAN 4 FEET SHALL BE BLOCKED BEYOND THE HEADERS AND METAL TIES NOT LESS THAN 16 GAGE BY 1-1/2" WIDE WITH (8) 16d COMMON NAILS ON EACH SIDE OF THE HEADER-JOIST INTERSECTION SHALL BE PROVIDED.

DIVISION 08.1 - PROTECTION OF OPENINGS

1. GLAZING IN BUILDINGS LOCATED WITHIN 1 MILE OF THE COASTAL MEAN HIGH WATER LINE WHERE THE ULTIMATE DESIGN WIND SPEED V(ULT) IS 130 OR GREATER OR IN AREAS WHERE THE ULTIMATE DESIGN WIND SPEED V(ULT) IS 140 MPH OR GREATER SHALL BE IMPACT RESISTANT OR PROTECTED WITH AN IMPACT RESISTANT COVERING MEETING THE REQUIREMENTS OF AN APPROVED IMPACT RESISTANT STANDARD OR ASTM E 1886 AND ASTM E 1996 AND AS FOLLOWS:

a. GLAZED OPENINGS LOCATED WITHIN 30 FEET OF GRADE SHALL MEET THE REQUIREMENTS OF THE LARGE MISSILE TEST OF ASTM E 1996.

GLAZED OPENINGS LOCATED MORE THAN 30 FEET ABOVE GRADE SHALL MEET THE PROVISIONS OF THE SMALL MISSILE TEST OF ASTM E 1996.

DIVISION 31.1 - EARTHWORK AND FOUNDATIONS 1. FOUNDATIONS ARE DESIGNED FOR THE ALLOWABLE SOIL BEARING PRESSURES LISTED BELOW. THE OWNER SHALL BE RESPONSIBLE FOR ENGAGING A TESTING AGENCY TO VERIFY THE SOIL BEARING CAPACITY AND

PROVIDE A GEOTECHNICAL INVESTIGATION PER THE REQUIREMENTS OF SECTION 1803 OF THE IBC. a. BUILDING FOUNDATION LOAD-BEARING VALUES (REF 2021 IBC §1806.2):

 VERTICAL FOUNDATION PRESSURE: 2000 PSF NET COEFFICIENT OF FRICTION: 0.25 WEIGHT OF SOIL: 110 PCF

MASONRY RETAINING WALL LOAD-BEARING VALUES: VERTICAL FOUNDATION PRESSURE: 2000 PSF NET AT REST EQUIVALENT FLUID PRESSURE: 60 PSF/FT LATERAL BEARING PRESSURE: 330 PSF/FT BELOW GRADE

 COEFFICIENT OF FRICTION: 0.40 5. CONTRACTOR SHALL OBTAIN A COPY OF THE SOILS REPORT AND ADHERE TO ALL RECOMMENDATIONS WITHIN, INCLUDING PREPARATION OF SOILS AT BUILDING PAD.

a. FILLS PLACED BENEATH THE SLAB OR FOOTINGS SHALL BE COMPACTED PER THE RECOMMENDATIONS OF A GEOTECHNICAL ENGINEER. BEARING ELEVATIONS OF FOOTINGS SHALL BE APPROVED BY A GEOTECHNCIAL ENGINEER PRIOR TO

PLACING ANY CONCRETE. 6. ALL SOILS WORK, INCLUDING BACKFILL OF UTILITY TRENCHES AND THE VERIFICATION OF BEARING CAPACITY OF SAME SHALL BE UNDER THE DIRECTION OF A QUALIFIED SOILS ENGINEER. PROXIMITY OF UTILITY

TRENCHES TO BUILDING FOUNDATION SYSTEM SHALL BE AS APPROVED BY THE SOILS ENGINEER TO INSURE INTEGRITY OF THE BEARING SOILS. FLOOR SLABS SHALL BEAR ON 4 INCHES OF DRAINAGE LAYER. THIS DRAINAGE LAYER SHALL CONSIST OF COMPACTED GRAVEL OR CLEAN SAND. THE MOISTURE RETARDER SHALL BE PLACED BETWEEN THE

DRAINAGE LAYER AND THE SLAB. 8. NO FOUNDATION CONCRETE SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD OF ALL CONFLICTS THAT EXIST BETWEEN FOOTINGS AND UTILITIES PRIOR TO CONSTRUCTION.

10. BACKFILL BEHIND RETAINING WALLS SHALL BE WITH AN ENGINEERED FILL PER THE RECOMMENDATIONS OF A GEOTECHNICAL ENGINEER. 11. HEAVY EQUIPMENT SHOULD NOT BE ALLOWED WITHIN 8 FEET OF ANY EARTH RETAINING WALL. USE ONLY HAND OPERATED VIBRATORY COMPACTORS FOR COMPACTING BEHIND RETAINING WALLS.

ALL FOUNDATIONS, OR PORTIONS THEREOF, BELOW GRADE MAY BE EARTH FORMED BY NEAT EXCAVATIONS.

12. STEM WALLS RESTRAINED BY UPPER SLABS SHALL BE BRACED AGAINST OVERTURNING AND SLIDING DURING CONSTRUCTION. 13. UNLESS OTHERWISE SHOWN, ALL FOOTINGS SHALL BE CENTERED ON WALLS AND/OR COLUMNS.

14. THE CONTRACTOR SHALL DETERMINE THE EXTENT OF CONSTRUCTION DEWATERING REQUIRED FOR THE EXCAVATION. THE CONTRACTOR SHALL SUBMIT TO THE GEOTECHNICAL ENGINEER FOR REVIEW THE PROPOSED PLAN FOR CONSTRUCTION DEWATERING, PRIOR TO EXCAVATION.

15. FOOTINGS SHALL NOT BE PLACED ON FROZEN SUBGRADE OR IN STANDING WATER.

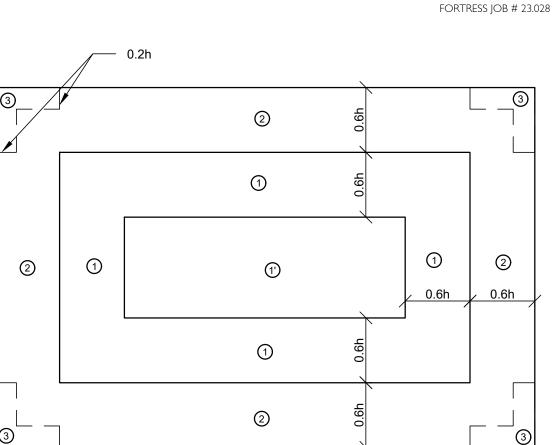
ABBREVIATIONS ARCH **ARCHITECTURAL** EOS **EDGE OF SLAB** LG **BOTTOM CHORD** EQ EQUAL LLH BLDG **BUILDING EACH WAY** LLV BEARING BRG **EXISTING** LLBB BTM BOTTOM EXP ANCH **EXPANSION ANCHOR** MAX **CENTER TO CENTER EXPANSION JOINT** MECH EXP JT EXT **EXTERIOR** CFSSF COLD FORMED STRUCTURAL MO STEEL FRAMING FDN **FOUNDATION CENTER LINE** FFE FINISHED FLOOR ELEVATION NIC FOC CLEAR COVER **FACE OF CONCRETE** NTS FRT COLUMN FIRE RETARDANT O.C. COL CONT **CONTINUOUS** FOM **FACE OF MASONRY** O/O FOS PAF DET DETAIL FACE OF STEEL DESIGN FLOOD ELEVATION GALV DFE GALVANIZED DWG **DRAWING** HORIZ HORIZONTAL HSB REF HIGH STRENGTH BOLT EACH FACE INT REINF INTERIOR ELEV REQ'D **ELEVATION** JST JOIST

S001 GENERAL NOTES S101 FOUNDATION/SLAB PLAN - HALL S102 ROOF FRAMING PLAN - HALL S103 FOUNDATION AND ROOF PLANS - CONCESSIONS S301 TYPICAL DETAILS S302 SECTIONS AND DETAILS S303 SECTIONS AND DETAILS S304 SECTIONS AND DETAILS S305 SECTIONS AND DETAILS S306 SECTIONS AND DETAILS



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<u>PLAN</u> WIND ZONE DIAGRAM AT ROOF, Θ ≤ 7° (TO BE USED WITH COMPONENTS AND CLADDING CHART

NAILING -UNO: FASTENING SCHEDULE RAFTER/ TRUSS TO TOP PLATE, TOENAIL (3) 10d CEILING JOIST TO TOP PLATE, TOENAIL 3. CEILING JOIST TO PARALLEL RAFTER, FACE NAIL CEILING JOIST LAPS OVER PARTITIONS, FACE NAIL 6. COLLAR TIE TO RAFTER, FACE NAIL (EACH END) (3) 10d BLOCKING TO RAFTER, TOENAIL (EACH END) BLOCKING TO TOP PLATE, TOENAIL (EACH BLOCK) RIM BOARD TO RAFTER, END NAIL 9. ROOF RAFTER TO RIDGE, VALLEY, OR HIP (4) 10d TOENAIL AND (3) 16d FACE NAIL (2) 16d PER FOOT 10. TOP PLATE TO TOP PLATE, FACE NAIL TOP PLATE AT INTERSECTIONS, FACE NAIL (4) 16d DOUBLE STUDS, FACE NAIL 2-ROWS 16d @ 24 OC CONTINUOUS HEADER, MULTIPLE PLY REF DET 3 & 5 SHT S302 14. DOUBLE TOP PLATES, LAP SPLICE REF DET 7 SHT S303 TOP OR BOTTOM PLATE TO STUDS, END NAIL (3) 16d at 2x6 (3) 16d PER STUD SPACE 16. BTM PLATE TO BAND JOIST OR BLOCKING, FACE NAIL 17. BTM PLATE TO FLOOR JOIST OR END JOIST, FACE NAIL (3) 16d PER STUD SPACE 18. JOIST TO SILL, TOP PLATE, OR GIRDER, TOENAIL (4) 8d 19. BRIDGING TO JOIST, TOENAIL (EACH END) (2) 8d 20. BLOCKING TO JOIST, TOENAIL (EACH END) (2) 8d 21. BLOCKING TO TOP PLATE, TOENAIL (EACH BLOCK) (3) 16d 22. LEDGER STRIP TO BEAM WITH MAX SPAN 12', FACE NAIL (EACH JOIST) (3) 16d 23. JOIST ON LEDGER OR BEAM, TOENAIL (3) 8d24. BAND JOIST TO JOIST, END NAIL (3) 16d25. BAND JOIST TO TOP PLATE, TOENAIL (3) 16d PER FOOT 26. BUILT-UP CORNER WITH 3 STUDS 2-ROWS 16d @ 6 OC 27. BUILT-UP CORNER WITH 4 STUDS 2-ROWS 16d @ 6 OC REF WOOD SHEATHING NOTES 28. 1x OR 2x DECKING

SEC

SHT

STD

T&B

TBV

TOM

TOS

TOW

TYP

UNO

VERT

VIF

WSP

WWR

WP

TOF

SECTION

SHEET

SIMILAR

STANDARD

TOP & BOTTOM

TOP OF STEEL

WORKING POINT

TOP OF WALL

TYPICAL

VERTICAL VERIFY IN FIELD

TO BE VERIFIED

TOP OF FOOTINGS

TOP OF MASONRY

UNLESS NOTED OTHERWISE

WOOD STRUCTURAL PANEL

WELDED WIRE REINFORCING

LONG

MAXIMUM

MINIMUM

MECHANICAL

NOT IN CONTACT

POWDER ACTUATED

FASTENER

REINFORCE / REINFORCING

PRESSURE TREATED

NOT TO SCALE

ON CENTER

OUT TO OUT

REFERENCE

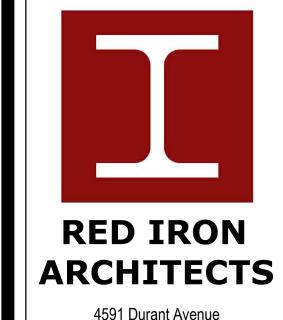
REQUIRED

LONG LEG HORIZONTAL

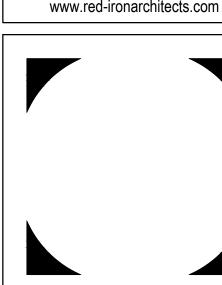
LONG LEG BACK TO BACK

LONG LEG VERTICAL

MASONRY OPENINGS



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DESCRIPTION

GENERAL NOTES

21081 Project Number: MAY 19, 2023 Drawn Bv: mjym/CJM

SECTION 1705.3 -- CONCRETE CONSTRUCTION FREQUENCY OF INSPECTION REFERENCED STANDARD^a REFERENCE CONTINUOUS PERIODIC ACI 318: CH. 20, 25.2, 25.3, INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT. 26.6.1-26.6.3 WELDING OF REINFORCING BARS: -A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706 Χ AWS D1.4, ACI 318: 26.6.4 Χ B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" C. INSPECT ALL OTHER WELDS Χ 3. INSPECT ANCHORS CAST IN CONCRETE Χ ACI 318: 17.8.2 4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS^b A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS ACI 318: 17.8.2.4 TO RESIST SUSTAINED TENSION LOADS. ACI 318: 17.8.2 B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a Χ ACI 318: CH. 19, 26.4.3, 26.4.4 1904.1, 1904.2, 5. VERIFY USE OF REQUIRED DESIGN MIX. 6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM ASTM C172, ASTM C31, ACI 318: 26.5, 26.12 SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION ACI 318: 26.5 **TECHNIQUES** Χ ACI 318: 26.5.3 - 26.5.5 8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES 9. INSPECT PRESTRESSED CONCRETE FOR: A. APPLICATION OF PRESTRESSING FORCES; AND ACI 318: 26.10 B. GROUTING OF BONDED PRESTRESSING TENDONS --10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS Χ ACI 318: CH. 26.9 11. FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOINTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MDE OR HDE) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F, INSPECT SUCH CONNECTIONS AND ACI 318: CH. 26.13.1.3 REINFORCEMENT IN THE FIELD FOR: ACI 550.5 A. INSTALLATION OF THE EMBEDDED PARTS B. COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS. -C. COMPLETION OF CONNECTIONS IN THE FIELD. 12. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR ACI 318: CH. 26.13.1.3 COMPLIANCE WITH ACI 550.5. 13. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED ACI 318: 26.11.2 CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL 14. INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER ACI 318: 26.11.1.2(b) WHERE APPLICABLE, SEE ALSO SECTION 1705.13, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE.

SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF WORK.

		SECTION 1705.4 MASONRY CONSTRUCTION			
		TYPE	FREQUENCY (a)	REFERENCE fo	or CRITERIA
		TIPE	LEVEL 2	TMS 402	TMS 602
1.	AS	MASONRY CONTRUCTIONS BEGINS VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
	A.	PROPORTIONS OF SITE-MIXED MORTAR	Р		Art. 2.1, 2.6 A, & 2.6 C
	B.	GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	Р		Art. 2.4 B, & 2.4 H
	C.	GRADE, TYPE, AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHO BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	Р		Art. 3.4, & 3.6 A
	D.	PRESTRESSING TECHNIQUE	Р		Art. 3.6 B
	E.	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	C ^(b) / P ^(c)		Art. 2.1 C.1
	F.	SAMPLE PANEL CONSTRUCTION	Р		Art. 1.6 D
2.	PR	RIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
	A.	GROUT SPACE	Р		Art. 3.2 D & 3.2 F
	B.	PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES	Р	Sec. 10.8 & 10.9	Art. 2.4 & 3.6
	C.	PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS	Р	Sec. 6.1, 6.3.1, 6.3.6, & 6.3.7	Art. 3.2 E & 3.4
	D.	PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	Р		Art. 2.6 B & 2.4 G.1.b
3.	VE	RIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:			
	A.	MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS	Р		Art. 1.5
	B.	PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	Р		Art. 3.3 B
	C.	SIZE AND LOCATION OF STRUCTURAL MEMBERS	Р		Art. 3.3 F
	D.	TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	Р	Sec. 1.2.1(e), 6.2.1	
	E.	WELDING OF REINFORCEMENT	С	Sec. 6.1.6.1.2	
	F.	PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPS BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPS ABOVE 90°F (32.2°C))	Р		Art. 1.8 C & 1.8 D
	G.	APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	С		Art. 3.6 B
	Н.	PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE.	С		Art. 3.5 & 3.6 C
	I.	PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	C ^(b) / P ^(c)		Art. 3.3 B.9 & 3.3 F.1
4.	OE	SSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMEN, AND/OR PRISMS	Р		Art. 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, & 1.4 B.4

a. FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE LISTED TASK OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE

NR = NOT REQUIRED, P = PERIODIC, C = CONTINUOUS

b. REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY. c. REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF AAC MASONRY

	SECTION 1705.6 SOILS			
	TVDE	FREQUENCY C	F INSPECTION	DEFEDENCED OTANDARD
	TYPE	CONTINUOUS	PERIODIC	REFERENCED STANDARD
1.	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	Х	
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	Х	
3.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	Х	
4.	DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	Х	-	
5.	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	Х	

STATEMENT OF SPECIAL INSPECTIONS:

- 1. THE STATEMENT OF SPECIAL INSPECTIONS OUTLINED IN THIS SECTION, AS SPECIFIED BY CHAPTER 17 OF THE 2021 IBC, REQUIRES THAT THE OWNER EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS LISTED IN THE TABLE ON THIS SHEET. A REPORT SHALL BE FURNISHED TO THE BUILDING OFFICIAL AND THE APPROPRIATE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE UPON THE COMPLETION EACH INSPECTION. UPON COMPLETION OF ALL SPECIAL INSPECTIONS A FINAL REPORT DOCUMENTING THE REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.
- 2. CONTRACTOR SHALL SUBMIT WRITTEN STATEMENT OF RESPONSIBILITY ACCORDING TO THE REQUIREMENTS LISTED IN SECTION 1704.4 OF THE IBC TO THE BUILDING OFFICIAL, OWNER,
- 3. ALL STRUCTURAL COMPONENTS AND STRUCTURAL SYSTEMS SHALL BE TESTED AND INSPECTED ACCORDING TO THE APPROPRIATE CODE SPECIFICATIONS LISTED IN THE TABLES ON
- 4. WORK DONE IN FABRICATOR SHOP REQUIRES INSPECTOR UNLESS THE FABRICATOR IS REGISTERED AND APPROVED ACCORDING TO IBC 1704.2.5.1. WHERE FABRICATOR IS APPROVED, PROVIDE FABRICATOR CERTIFICATION DOCUMENT. AT COMPLETION OF FABRICATION, SUBMIT CERTIFICATE OF COMPLIANCE TO BUILDING OFFICIAL STATING WORK WAS PERFORMED
- IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. 5. SPECIAL INSPECTIONS NOTED AS "PERIODIC": THE INSPECTIONS AGENCY'S PART-TIME OR INTERMITTENT OBSERVATION OF WORK DURING CONSTRUCTION BY BEING PRESENT IN THE
- AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED, AND OBSERVATION UPON COMPLETION OF THE WORK. 6. SPECIAL INSPECTIONS NOTED AS "CONTINUOUS": THE INSPECTION AGENCY'S FULL-TIME OBSERVATION OF WORK BY BEING PRESENT IN THE AREA WHERE THE WORK IS BEING
- 7. STRUCTURAL OBSERVATIONS DURING CONSTRUCTION WILL NOT BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD UNLESS SPECIFICALLY REQUESTED BY THE CLIENT.
- 8. WIND AND SEISMIC FORCE RESISTING SYSTEM: SPECIAL REINFORCED MASONRY SHEAR WALLS AND CANTILEVERED TIMBER FRAMES

SECTION 1705.12.3 SPECIAL INSPECTIONS FOR WIND RESISTING COMPONENTS					
VEDICICATION AND INSPECTION	FREQUENCY C	F INSPECTION	REFERENCED STANDARD		
VERIFICATION AND INSPECTION		PERIODIC	REFERENCED STANDARD		
1. ROOF COVERING, ROOF DECK, AND ROOF FRAMING CONNECTIONS.	-	Х			
2. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING.	-	Х			

	TYPE FREQUENCY OF INSPECTION		REFERENCED STANDARD	
TIFE		CONTINUOUS	PERIODIC	INCI ENCINCED STANDAND
1.	INSPECTION OF FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC FORCE-RESISTING SYSTEM.	Х	-	
2.	INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLD-DOWNS.	-	Х	

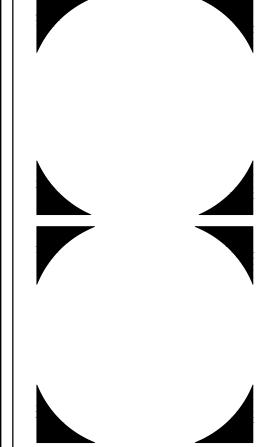
SECTION 1705.13.5 ARCHITECTURAL COMPONENTS FOR SEISMIC RESISTANCE AND	SECTION	1705.13.6 MEP	COMPONENTS FOR SEISMIC RESISTANCE
DECLUDED VERIFICATION AND INCRECTION	FREQUENCY OF INSPECTION		IDC DEFEDENCE
REQUIRED VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	IBC REFERENCE
ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS, AND INTERIOR AND EXTERIOR VENEER IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E, OR F.	-	Х	1705.13.5
ANCHORAGE OF ELECTRICAL EQUIPMENT FOR EMERGENCY AND STANDBY POWER SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F. ANCHORAGE OF OTHER ELECTRICAL EQUIPMENT IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY E OR F. INSTALLATION AND ANCHORAGE OF PIPING SYSTEMS DESIGNED TO CARRY HAZARDOUS MATERIALS AND THEIR ASSOCIATED MECHANICAL UNITS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F. INSTALLATION AND ANCHORAGE OF DUCTWORK DESIGNED TO CARRY HAZARDOUS MATERIALS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F. INSTALLATION AND ANCHORAGE OF VIBRATION ISOLATION SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F WHERE THE APPROVED CONSTRUCTION DOCUMENTS REQUIRE A NOMINAL CLEARANCE OF 1/4 INCH (6.4 MM) OR LESS BETWEEN THE EQUIPMENT SUPPORT FRAME AND RESTRAINT. INSTALLATION OF MECHANICAL AND ELECTRICAL EQUIPMENT, INCLUDING DUCT WORK, PIPING SYSTEMS AND THEIR STRUCTURAL SUPPORTS, WHERE AUTOMATIC SPRINKLER SYSTEMS ARE INSTALLED IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F TO VERIFY ONE OF THE FOLLOWING: 6.1. MINIMUM CLEARANCES HAVE BEEN PROVIDED AS REQUIRED BY SECTION 13.2.3 ASCE/SEI 7. 6.2. A NOMINAL CLEARANCE OF NOT LESS THAN 3 INCHES (76 MM) HAS BEEN BE PROVIDED BETWEEN AUTOMATIC SPRINKLER SYSTEMS AND SPRIGS AND: STRUCTURAL MEMBERS NOT USED COLLECTIVELY OR INDEPENDENTLY TO SUPPORT THE SPRINKLERS; EQUIPMENT ATTACHED TO THE BUILDING STRUCTURE: AND OTHER SYSTEMS' PIPING.	-	X	1705.13.6



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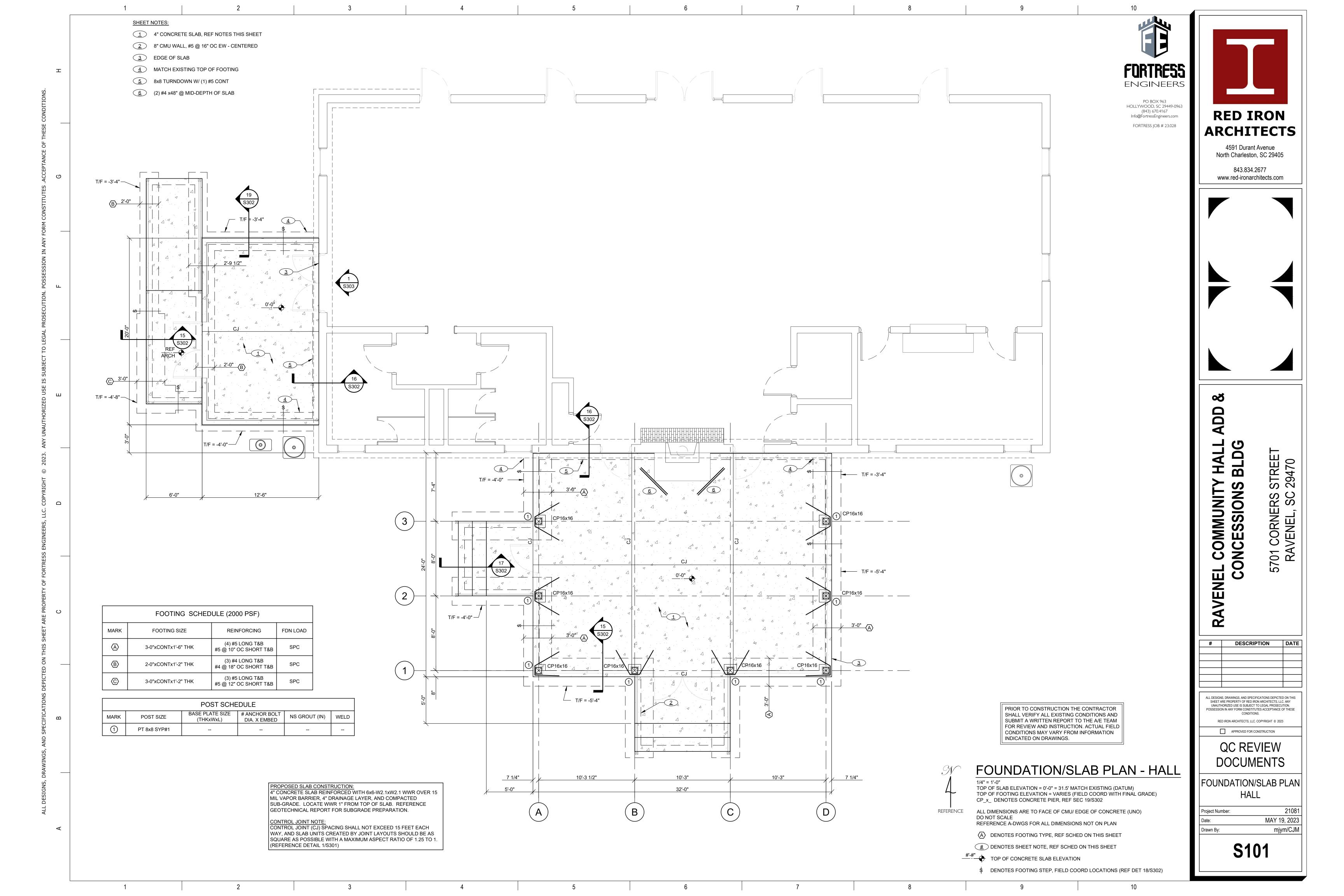
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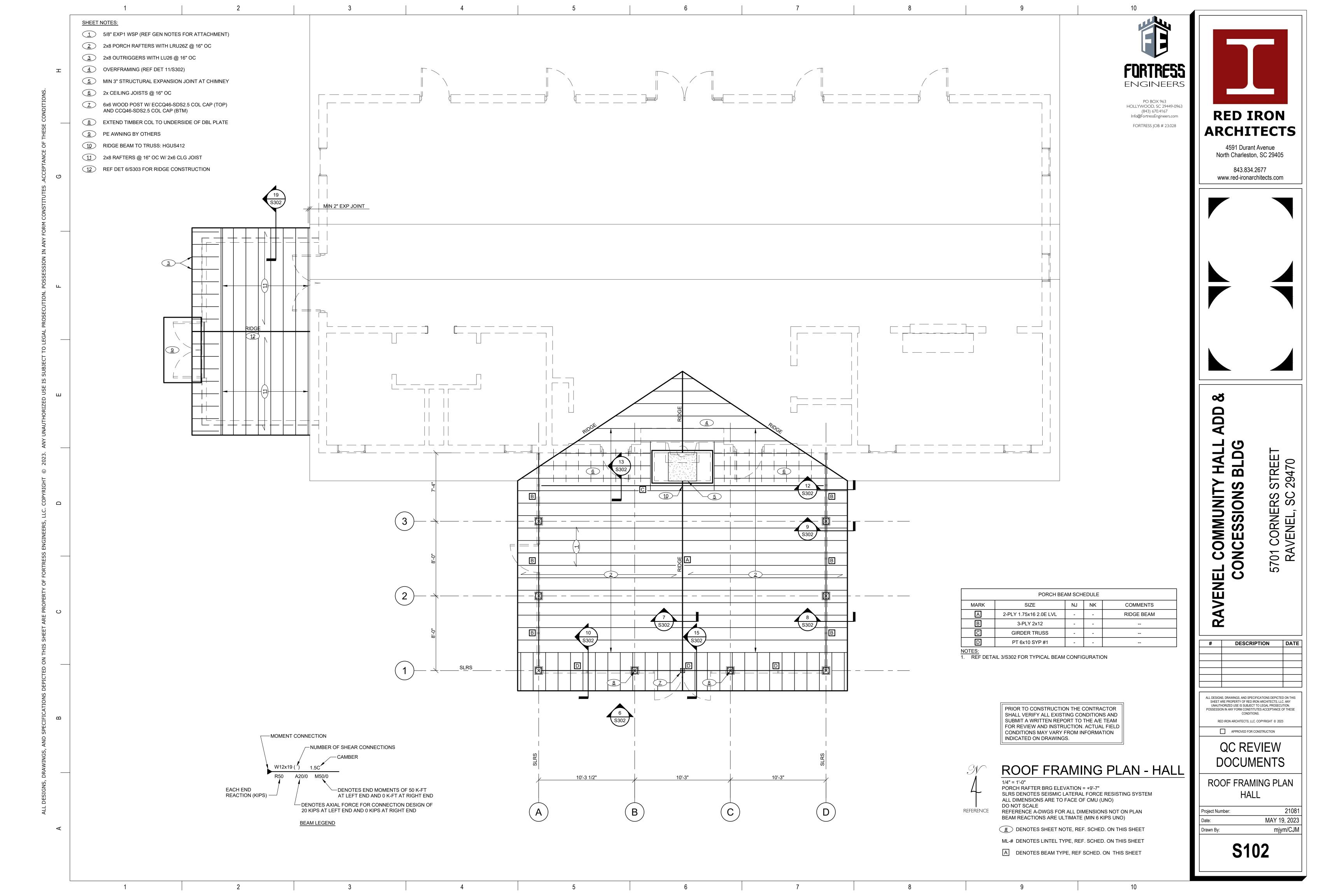
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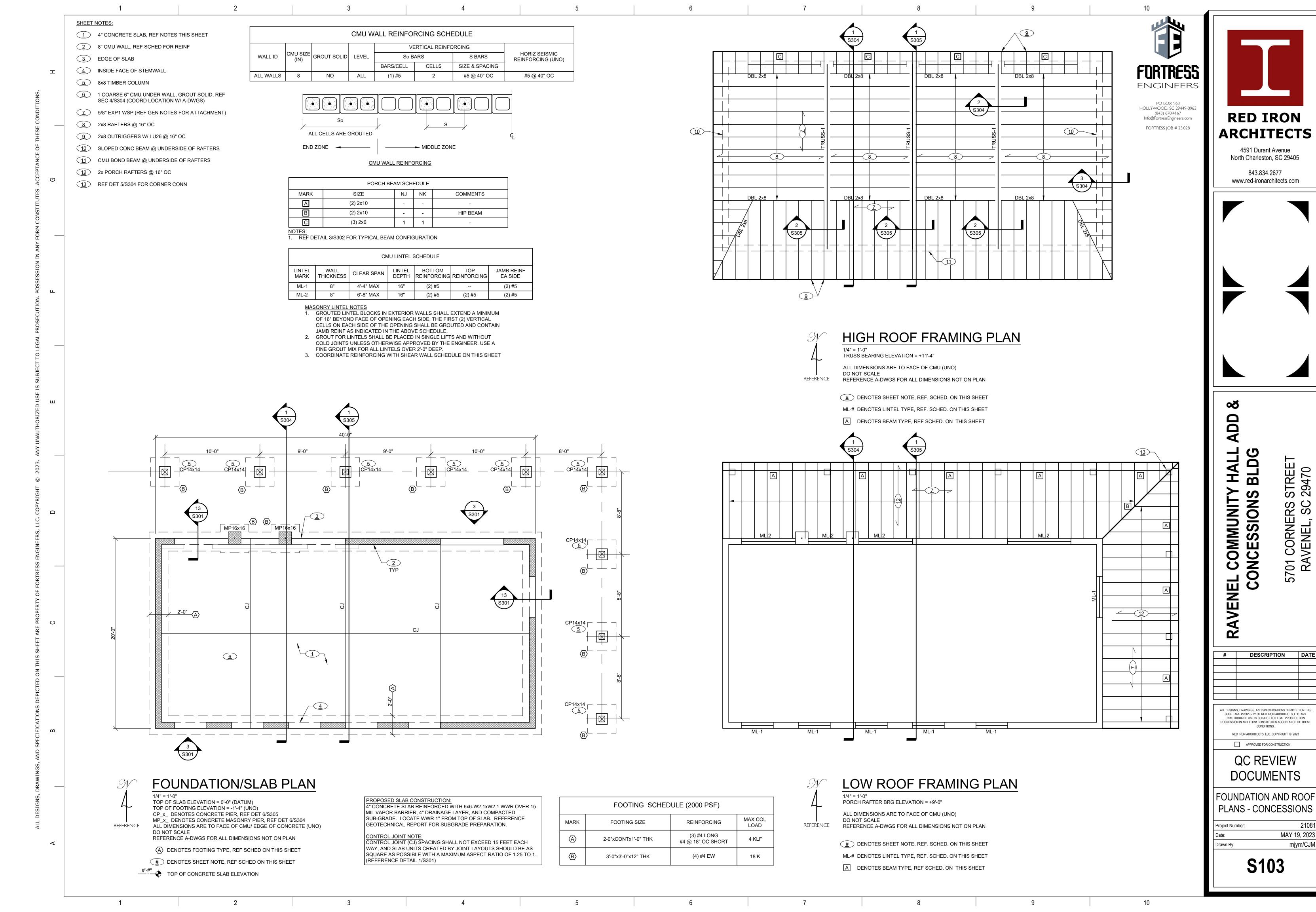
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QUALITY ASSURANCE PLAN

Project Nur	mber: 21081
Date:	MAY 19, 2023
Drawn By:	mjym/CJM



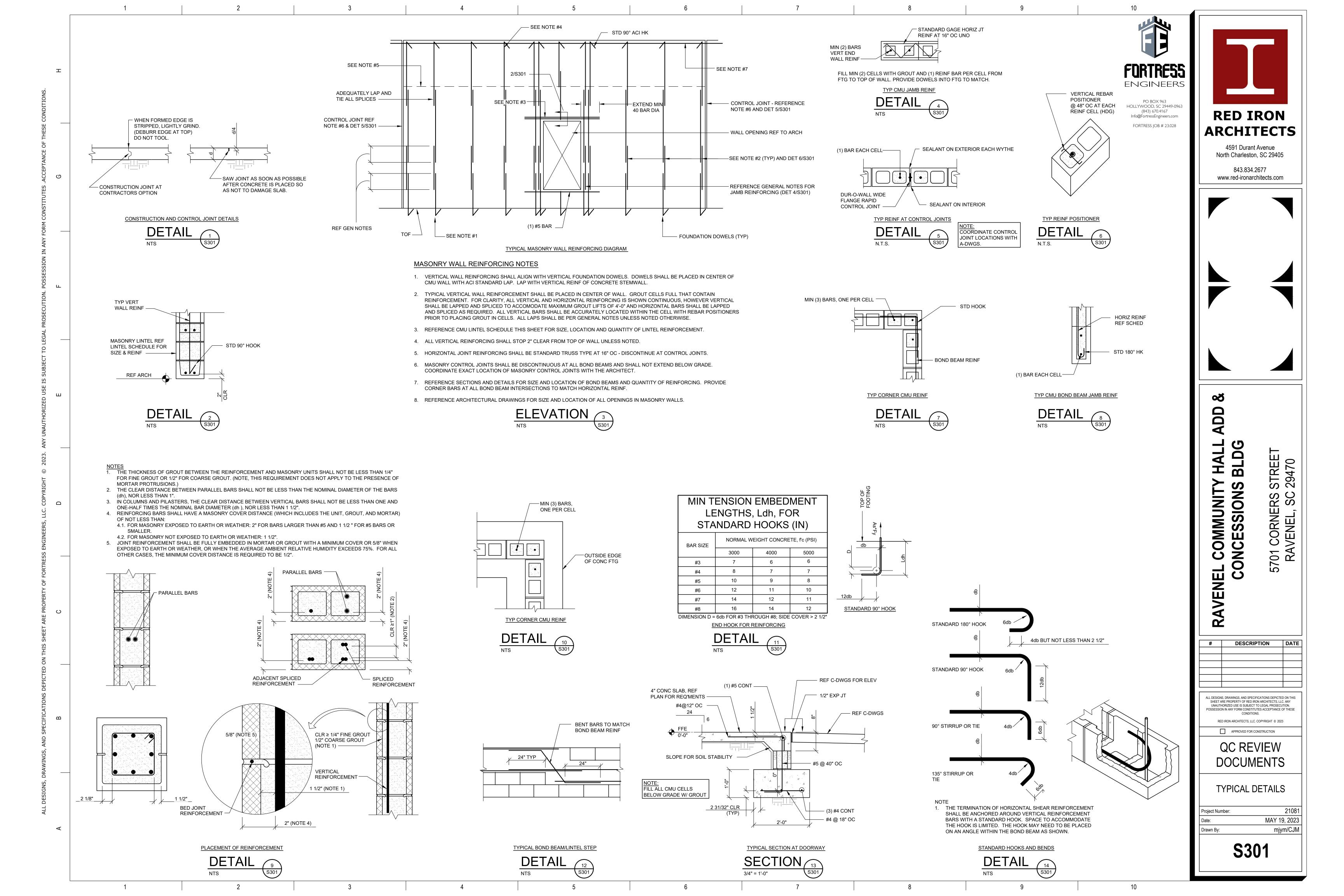


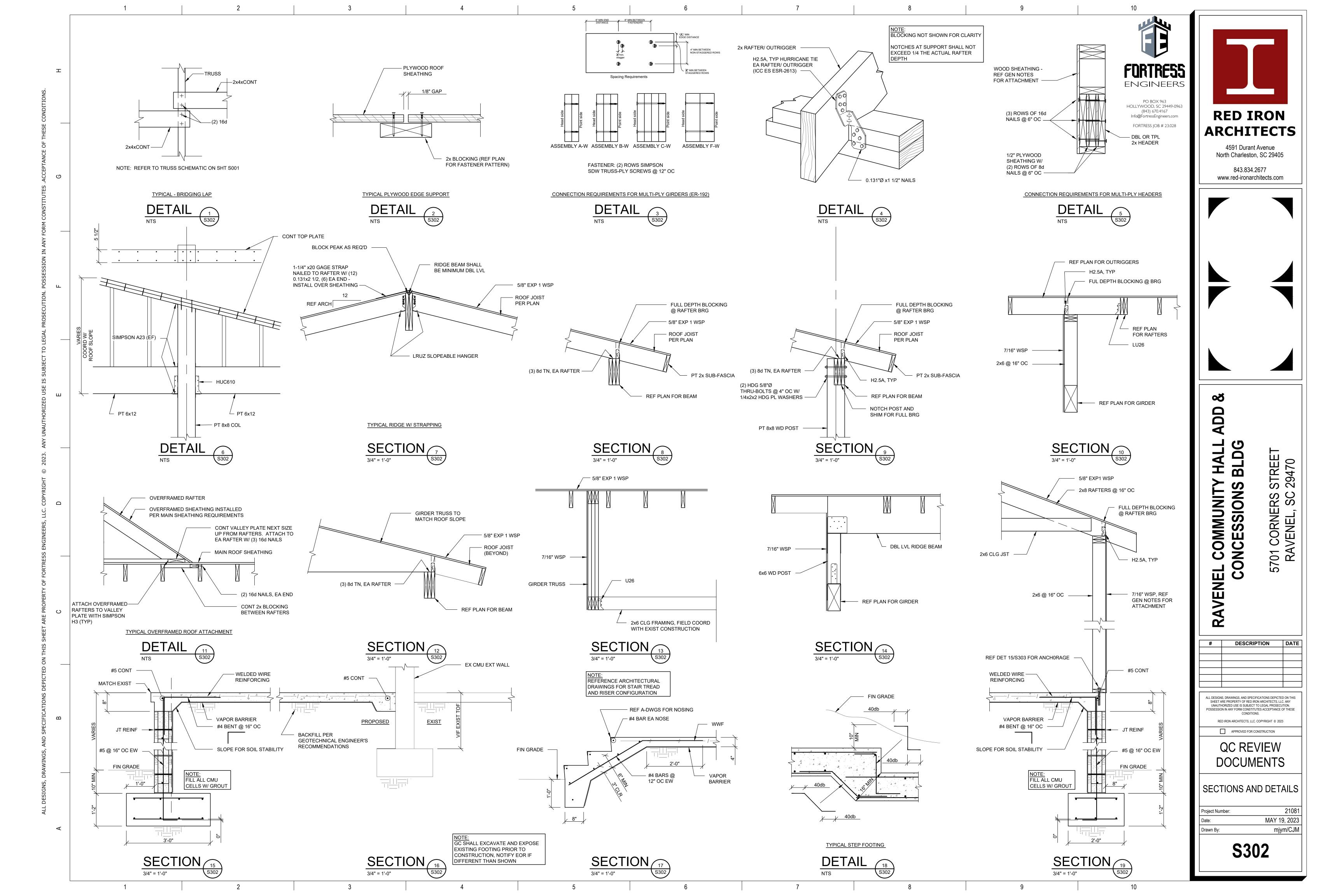


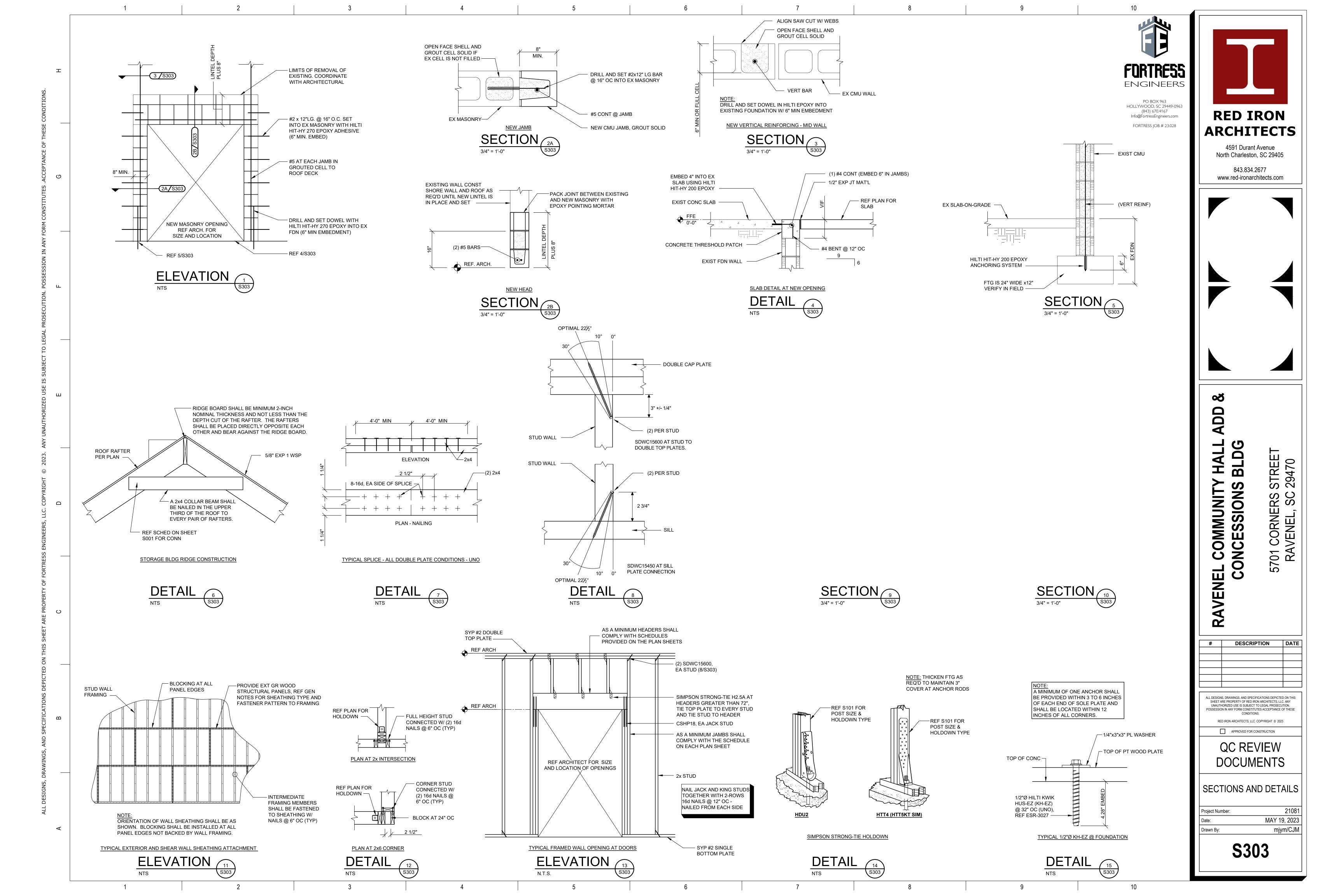
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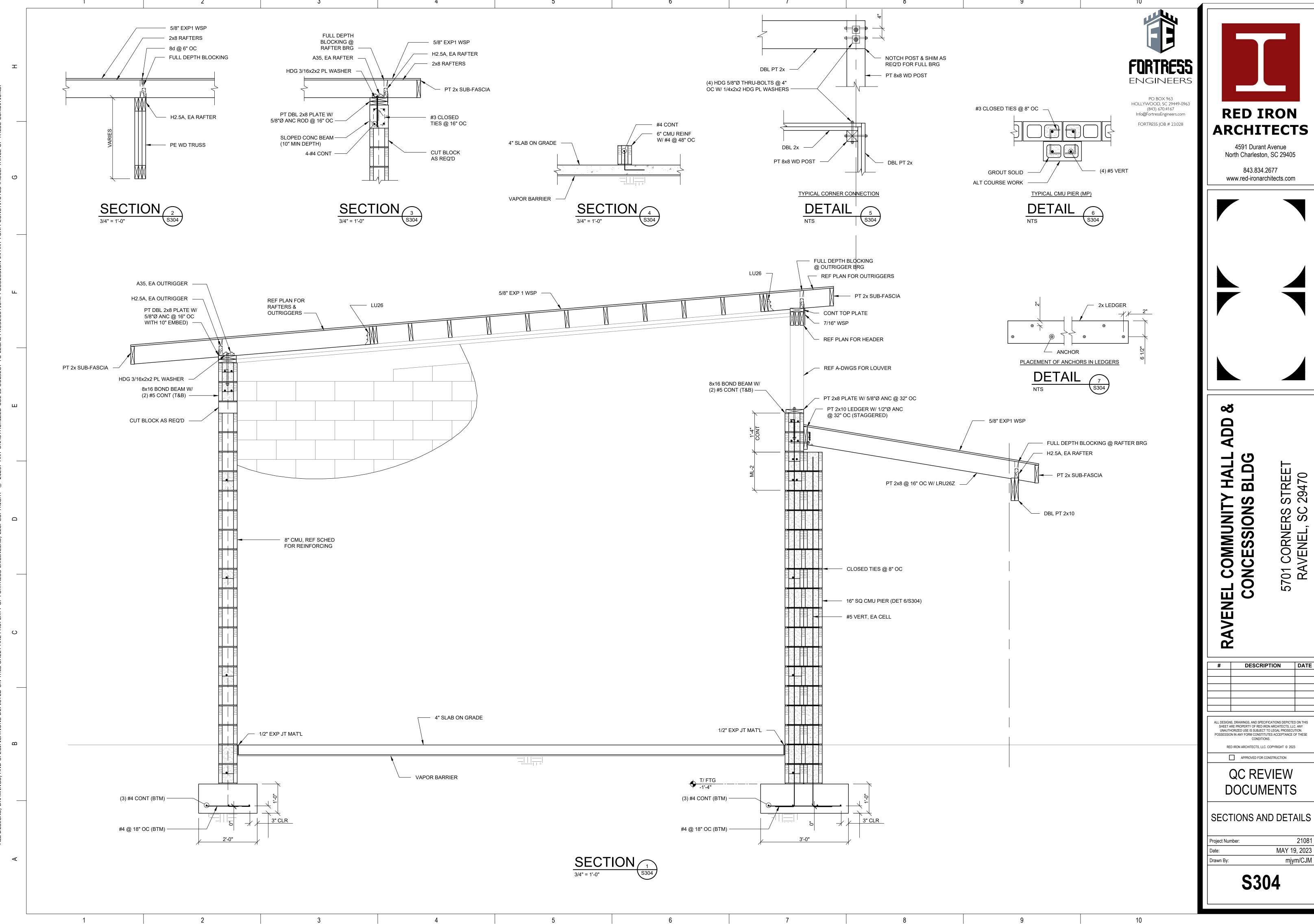
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mjym/CJM





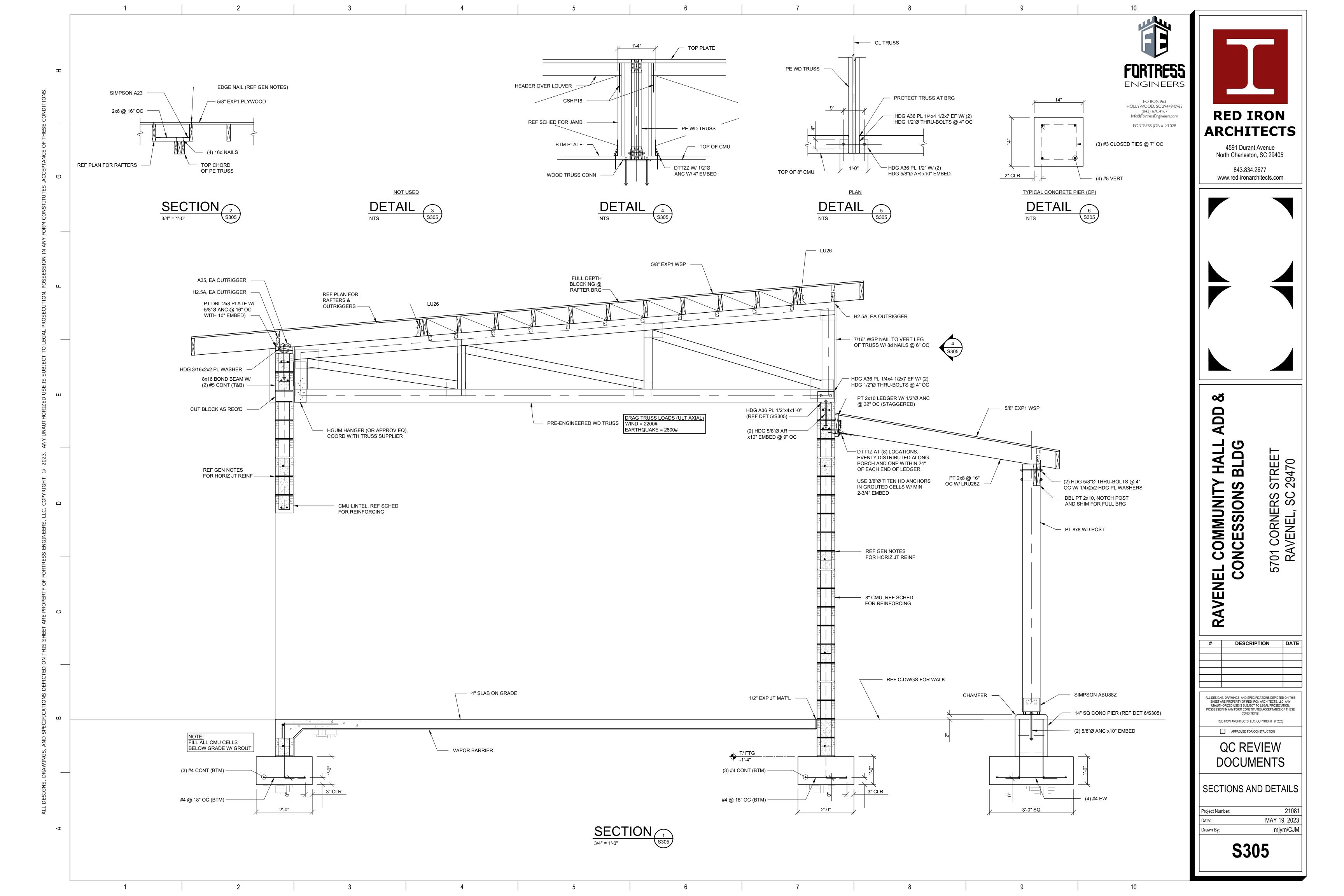


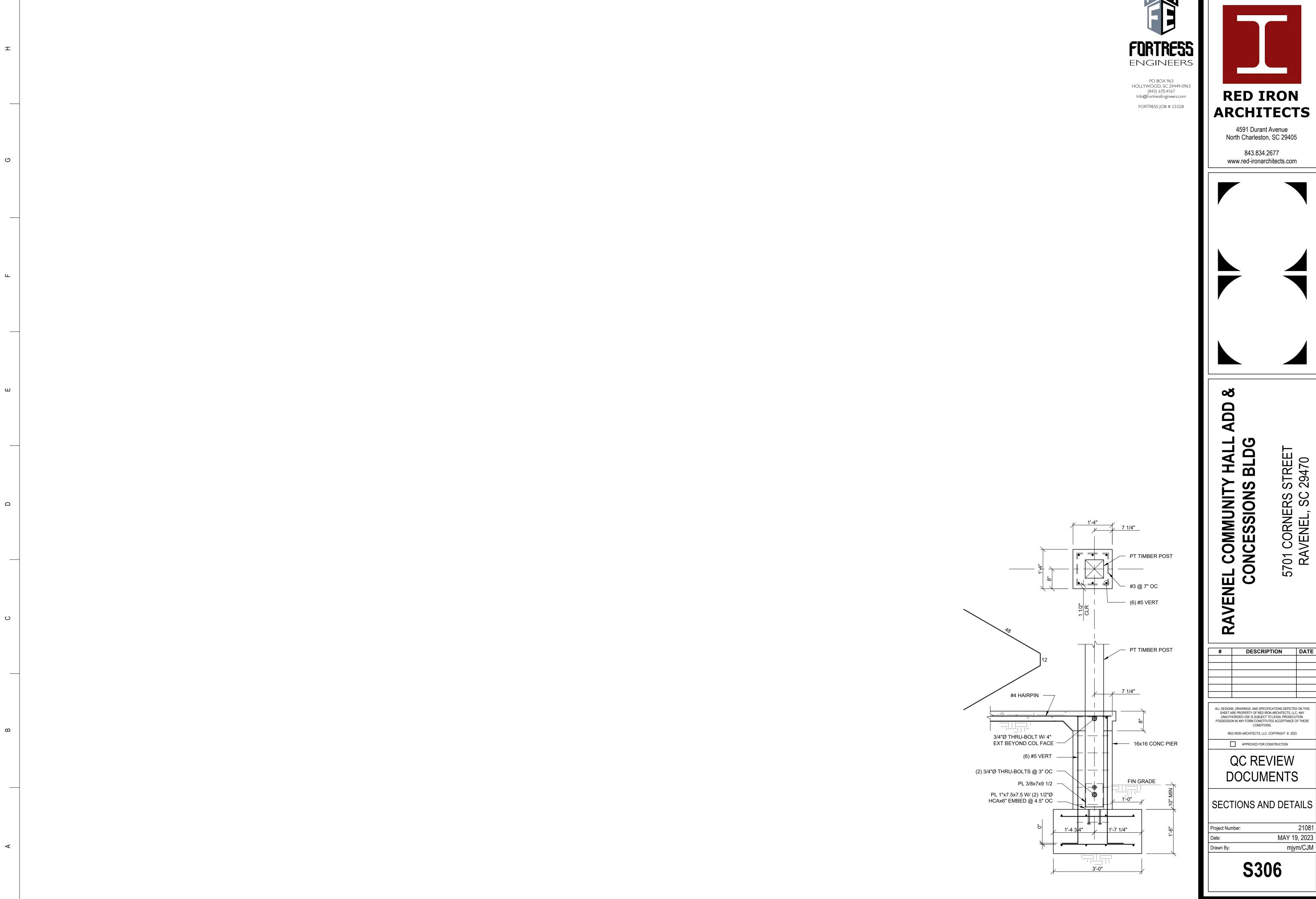


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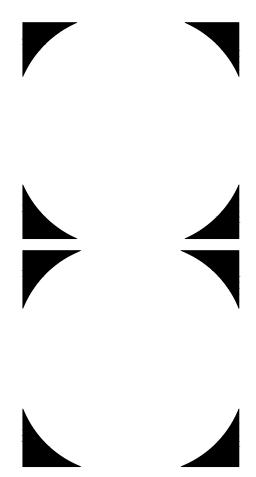
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MAY 19, 2023 mjym/CJM









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- 1. PRIOR TO CONSTRUCTION CONTRACTOR TO FIELD VERIFY ALL SITE AND BUILDING LAYOUT DIMENSIONS AND NOTIFY ARCHITECT OF ANY
- DISCREPANCY IMMEDIATELY.
- 2. DEMOLITION INCLUDES HAULING AND PROPER DISPOSAL. 3. REMOVE ALL DOORS, FRAMES AND DOOR HARDWARE NOT NOTED TO REMAIN AND RETURN TO OWNER.
- 4. ALL AREAS NOT NOTED FOR DEMOLITION ARE TO BE PROTECTED.

DEMOLITION LEGEND

EXISTING DOOR AND DOOR FRAME TO BE REMOVED COMPLETE U.N.O.

DEMOLITION KEYNOTES

NOT USED 2 NOT USED

3 NOT USED

- A REMOVE WINDOW, WINDOW FRAME, AND PORTION OF WALL COMPLETE FOR NEW DOOR OPENING
- 5 PORTION OF RAFTERS, SOFFIT, AND ROOF TO BE REMOVED FOR NEW SCREEN PORCH ADDITION



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DEMOLITION PLAN -HALL

21081 Project Number: APRIL 19, 2023 Drawn By: Author

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D1.01

#1 PICTURE 4

A1 HALL - DEMO PLAN D1.01 3/16" = 1'-0"

#2 PICTURE

PLAN & TRUE NORTH



#3. SIDE VIEW @ PROPOSED ADDITION

DEMOLITION NOTES

THIS PLAN REPRESENTS EXISTING CONDITIONS. IT IS INTENDED FOR REFERENCE ONLY. THIS PLAN IS NOT INTENDED TO BE A DESCRIPTION OF ALL REQUIRED WORK.

- . PRIOR TO CONSTRUCTION CONTRACTOR TO FIELD VERIFY ALL SITE AND BUILDING LAYOUT DIMENSIONS AND NOTIFY ARCHITECT OF ANY
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- REMAIN AND RETURN TO OWNER. 4. ALL AREAS NOT NOTED FOR DEMOLITION ARE TO BE PROTECTED.

DEMOLITION LEGEND

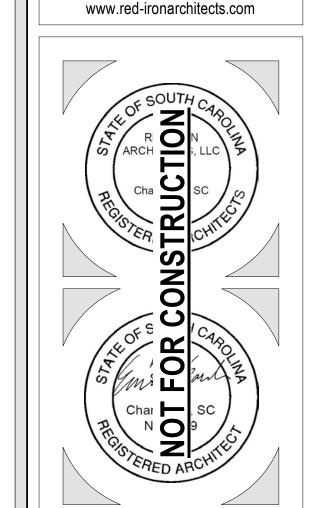
EXISTING DOOR AND DOOR FRAME TO BE REMOVED COMPLETE U.N.O.

DEMOLITION KEYNOTES

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PLAN & TRUE NORTH

- 3 NOT USED
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- 5 PORTION OF RAFTERS, SOFFIT, AND ROOF TO BE REMOVED FOR NEW SCREEN PORCH ADDITION



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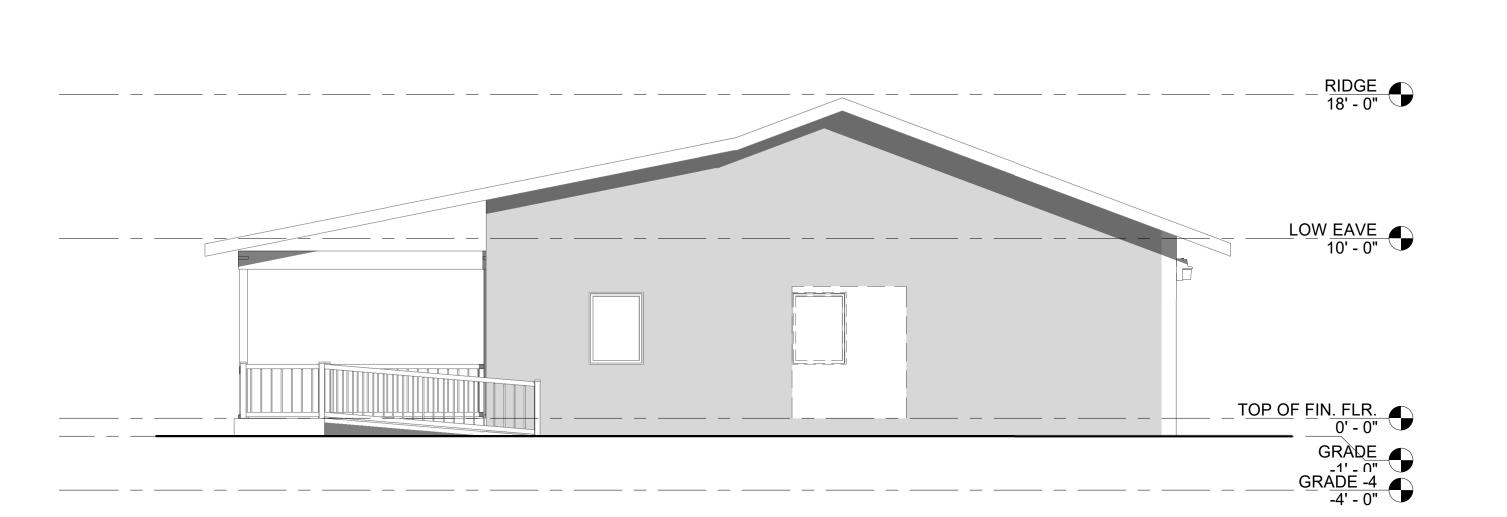
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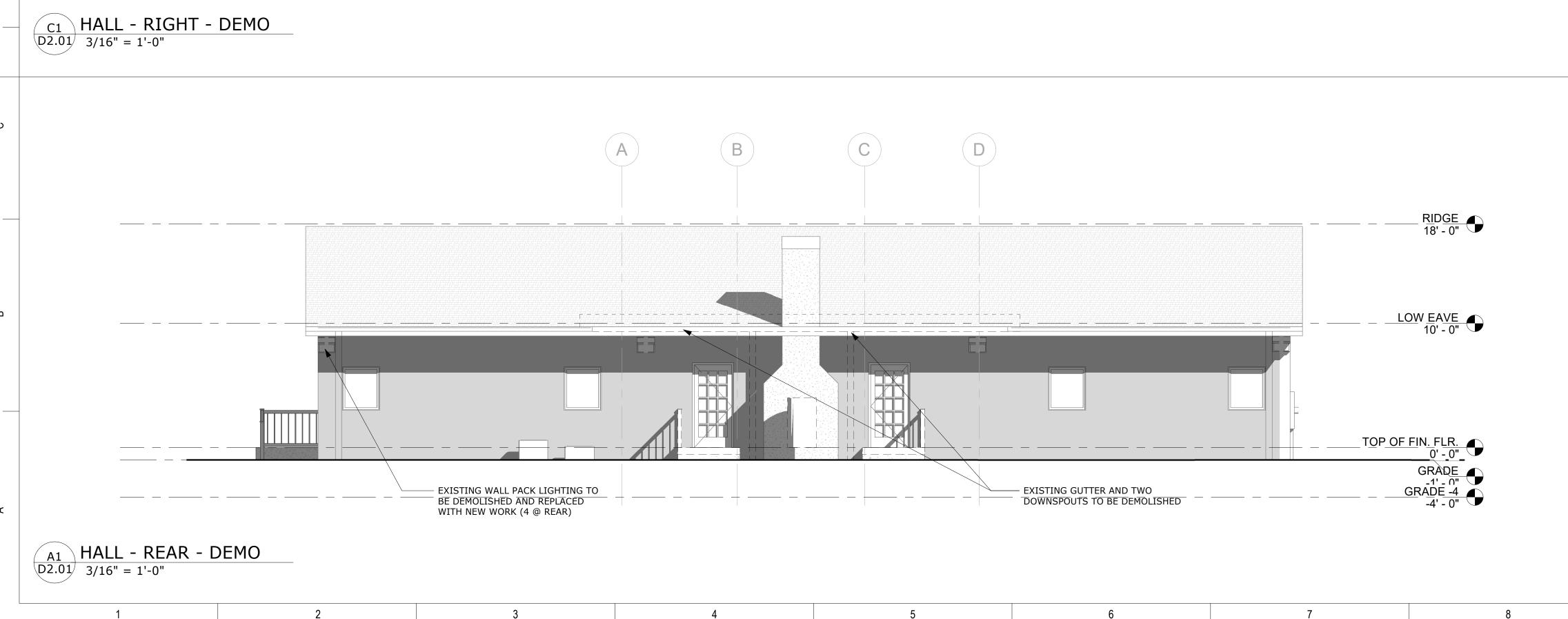
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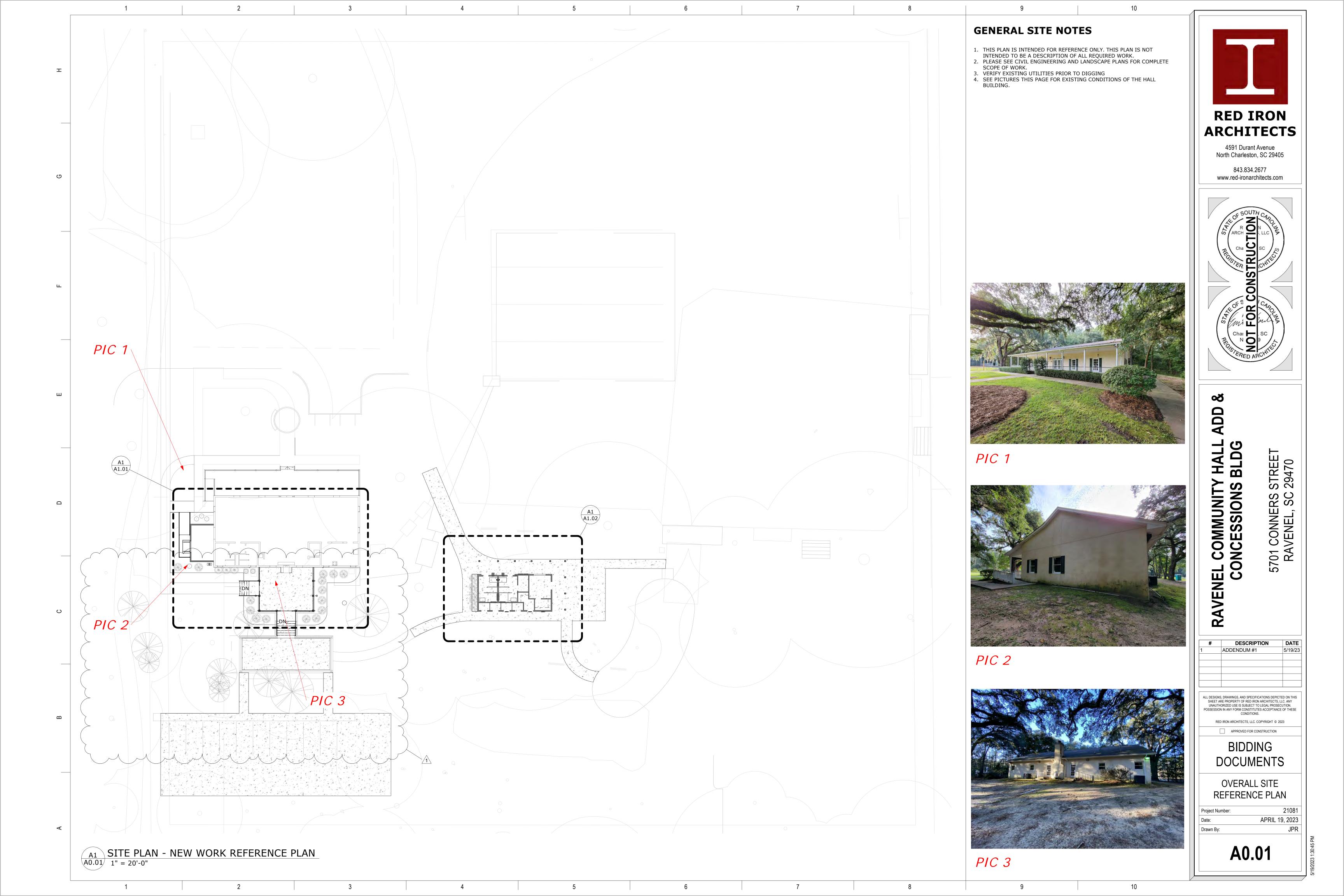
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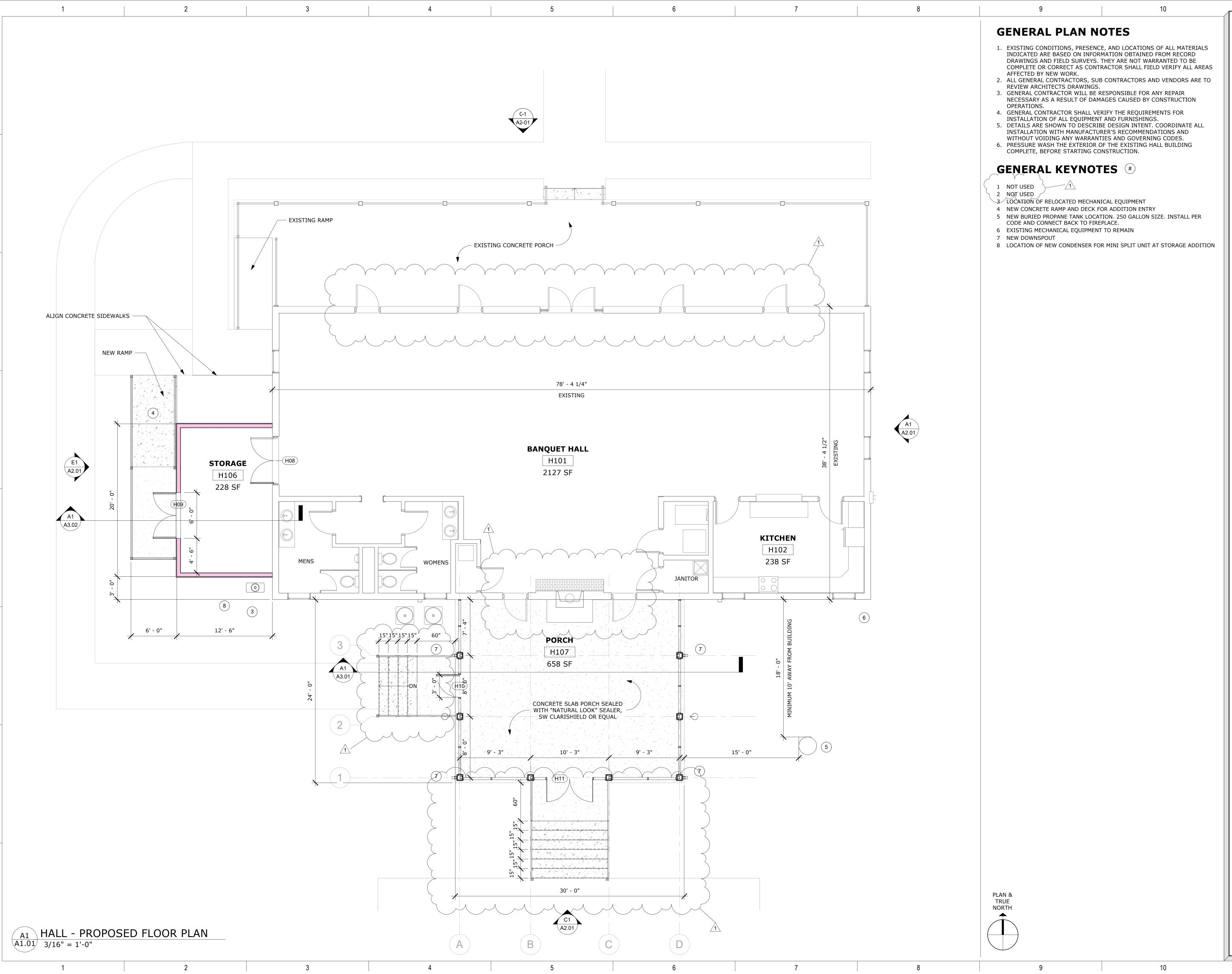
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D2.01





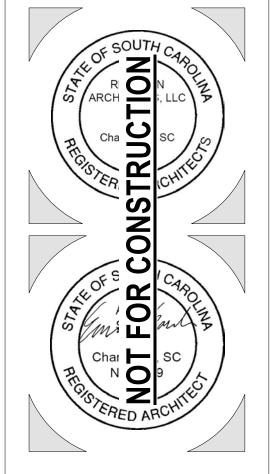




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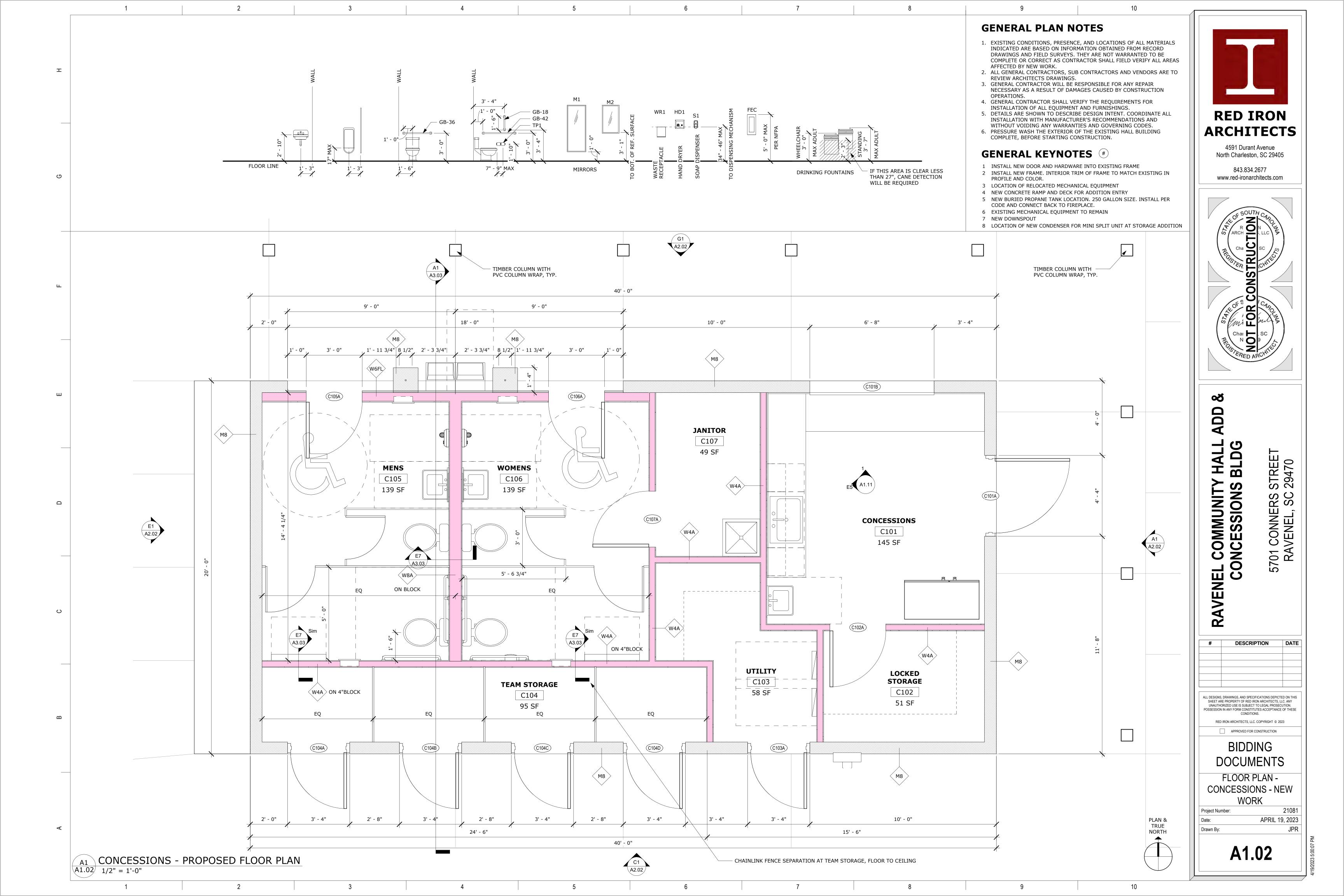
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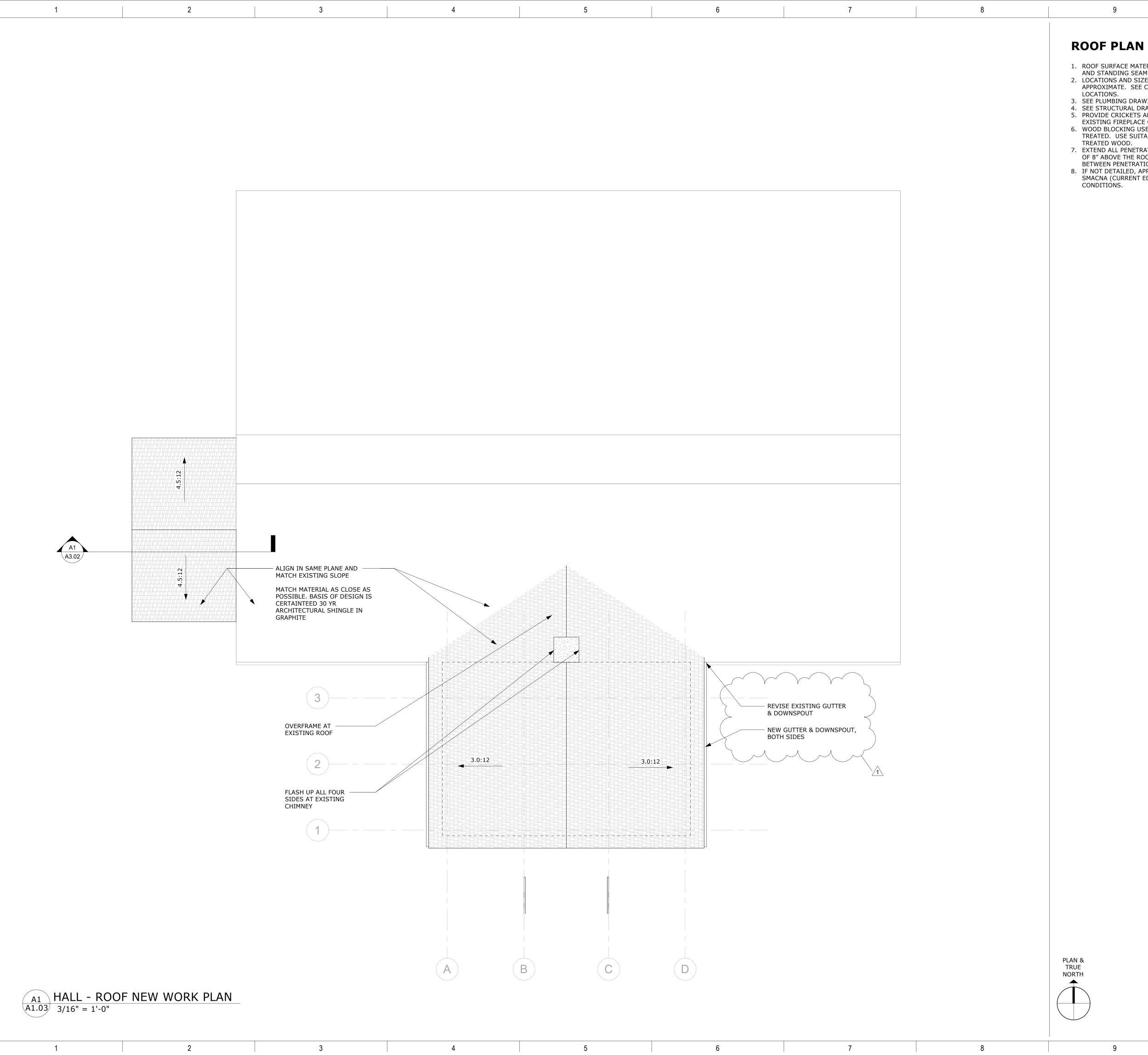
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FLOOR PLAN - HALL -**NEW WORK**

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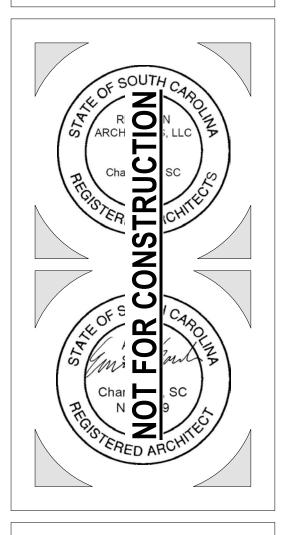
ROOF PLAN NOTES

- 1. ROOF SURFACE MATERIAL IS NOTED ON PLANS SHINGLE ON HALL
- AND STANDING SEAM ON CONCESSIONS BUILDING. 2. LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT ARE APPROXIMATE. SEE CONSULTANT DRAWINGS FOR ACTUAL
- 3. SEE PLUMBING DRAWINGS FOR VENTS THROUGH ROOF LOCATIONS. 4. SEE STRUCTURAL DRAWINGS FOR FASTENING REQUIREMENTS.
- 5. PROVIDE CRICKETS AND ADEQUATE FLASHING AT ALL CURBS AND EXISTING FIREPLACE CHIMNEY.
- 6. WOOD BLOCKING USED IN ROOF DETAILING SHALL BE PRESERVATIVE TREATED. USE SUITABLE FASTENERS WITH THE PRESERVATIVE
- 7. EXTEND ALL PENETRATIONS, CURBS, AND COMPONENTS A MINIMUM OF 8" ABOVE THE ROOF SURFACE. PROVIDE 12" CLEAR DISTANCE
- BETWEEN PENETRATIONS AND TO ANY VERTICAL SURFACE. 8. IF NOT DETAILED, APPLY MOST STRINGENT CONDITION OF NRCA AND SMACNA (CURRENT EDITIONS) STANDARDS FOR ROOF DETAIL



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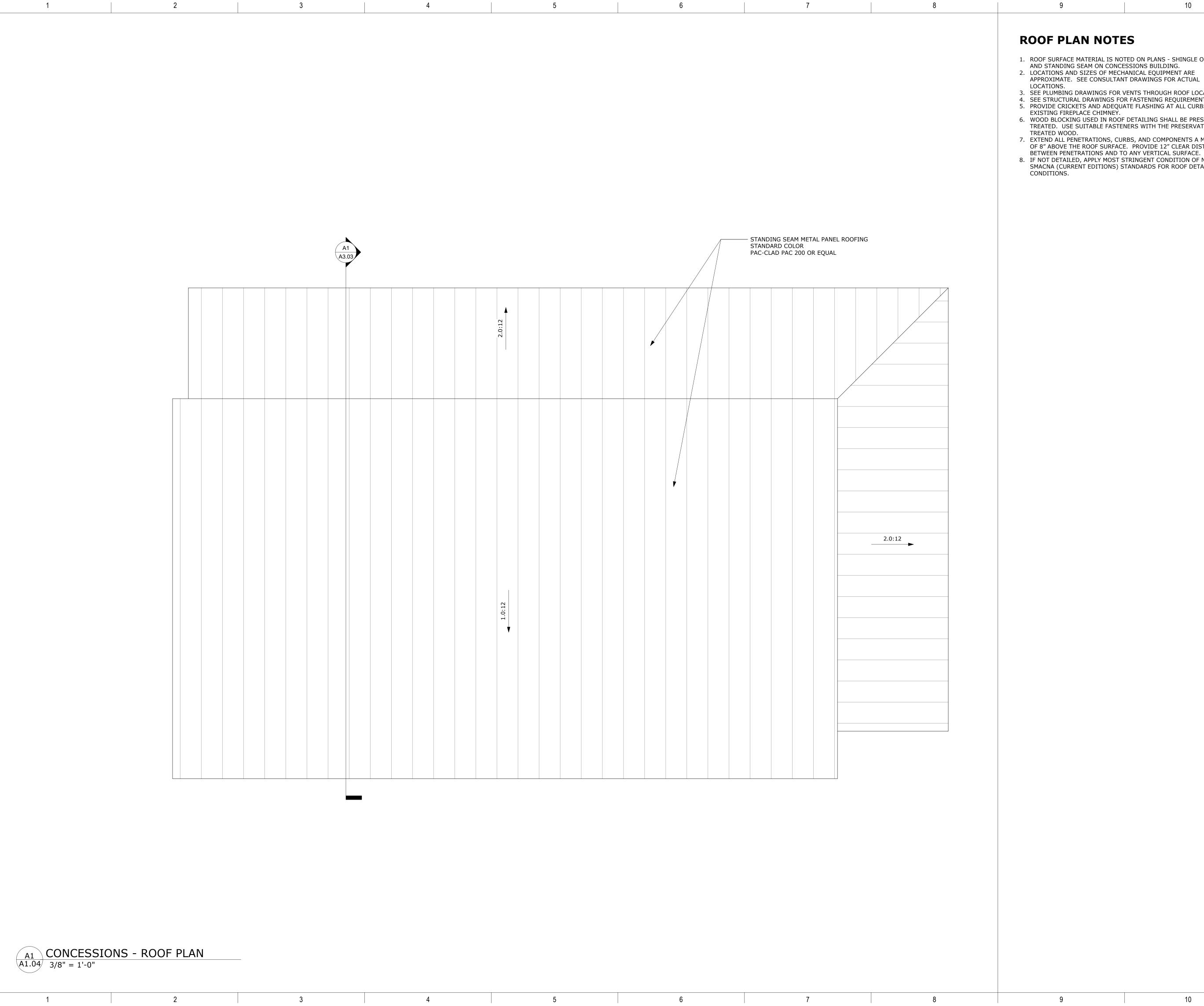
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ROOF PLAN - HALL

APRIL 19, 2023 Drawn By:

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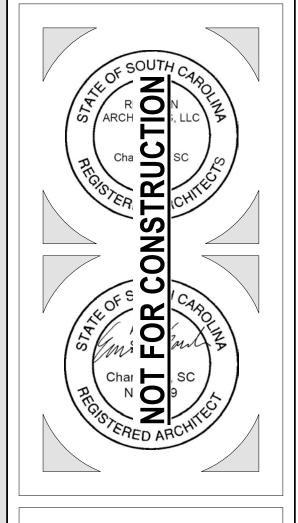


- 1. ROOF SURFACE MATERIAL IS NOTED ON PLANS SHINGLE ON HALL
- AND STANDING SEAM ON CONCESSIONS BUILDING. 2. LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT ARE
- 3. SEE PLUMBING DRAWINGS FOR VENTS THROUGH ROOF LOCATIONS. 4. SEE STRUCTURAL DRAWINGS FOR FASTENING REQUIREMENTS. 5. PROVIDE CRICKETS AND ADEQUATE FLASHING AT ALL CURBS AND
- 6. WOOD BLOCKING USED IN ROOF DETAILING SHALL BE PRESERVATIVE TREATED. USE SUITABLE FASTENERS WITH THE PRESERVATIVE
- 7. EXTEND ALL PENETRATIONS, CURBS, AND COMPONENTS A MINIMUM OF 8" ABOVE THE ROOF SURFACE. PROVIDE 12" CLEAR DISTANCE
- BETWEEN PENETRATIONS AND TO ANY VERTICAL SURFACE. 8. IF NOT DETAILED, APPLY MOST STRINGENT CONDITION OF NRCA AND SMACNA (CURRENT EDITIONS) STANDARDS FOR ROOF DETAIL



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ROOF PLAN -CONCESSIONS

21081 APRIL 19, 2023

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CEILING PLAN NOTES

- 1. SEE SHEET A1.20 FOR SEISMIC CEILING CONSTRUCTION DETAILS. THESE DETAILS ARE PROVIDED TO ILLUSTRATE THE REQUIREMENTS OF CISCA GUIDELINES FOR SEISMIC RESTRAINT AND IBC CHAPTER 16. GC IS RESPONSIBLE FOR MEETING ALL APPLICABLE PROVISIONS OF THE STANDARDS AND IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT OPTION.
- 2. CEILING GRIDS/TILES TO BE CENTERED IN ALL ROOMS UNLESS NOTED OTHERWISE. PARTIAL TILES AT ROOM PERIMETERS SHALL NOT BE LESS THAN 6" IN EITHER DIMENSION. TILES LESS THAN 6" IN LENGTH SHALL BE CUT FROM A 24"X48" TILE IN LIEU OF THE 24"X24"
- 3. ALL CEILINGS TO BE 9'-0" AFF, UNO. CEILING HEIGHTS SHOWN ON THE REFLECTED CEILING PLANS ARE NON-TYPICAL AND SPECIFIC TO
- THE AREA INDICATED. 4. LIGHT FIXTURES ARE SHOWN FOR POSITIONING IN FINISH CEILING SYSTEM. CENTER ALL FIXTURES WITHIN ROOM AND/OR CEILING TILE UNLESS SHOWN OTHERWISE.
- 5. SEE MPE PLANS FOR FIXTURES NOT INCLUDED ON THIS RCP, BUT PART OF THE PROJECT. COORDINATE WITH MPE DRAWINGS AND
- WHERE CONFLICTS OF LOCATION EXIST, REFER TO ARCHITECTURAL. 6. EXTEND PERIMETER WALLS AND GWB FINISH TO DECK AT WALLS WITHOUT A FINISHED CEILING SYSTEM. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR MOUNTING LOCATIONS OF ITEMS WHERE NO CEILING IS INDICATED.
- 7. ALL LIGHTS, DIFFUSERS, EXIT SIGNS, SMOKE DETECTORS, SPEAKERS, GENERAL ALARM SPEAKERS/STROBES, & MISC OTHER DEVICES IN CEILING TILES WHERE THEY ARE LOCATED. ALIGN MULTIPLE ITEMS ON CENTER OR EDGES.
- 8. INSTALL ACCESS PANELS IN GYPSUM BOARD CEILINGS AT DUCT DAMPER CONTROLS, DUCT MOUNTED SMOKE DETECTORS, MANUAL
- DUCT CONTROLS, ETC. SIZE PANEL OPENING FOR NEEDS. 9. LIGHTS LOCATED IN STAIRS SHALL OCCUR AT EACH FLOOR AND INTERMEDIATE LANDINGS.

RCP LEGEND

1'-0"

AFF

HEIGHT (FEET, INCHES) AFF

4" RECESSED CAN FIXTURE (WITH BATTERY BACKUP OPTION) & ALL DIMMABLE LIGHT FIXTURE SPEC: HELIOS LIGHTING - 4" ROUND DOWNLIGHT

4R-15L-35K-WF-EMI | 5 COUNT (EMERGENCY BATTERY BACKUP) 4R-15L-35K-WF | 7 COUNT

SURFACE MOUNTED 1X4 LED FIXTURE FOR STORAGE

BRACKET LIGHT - 12" SHADE LIGHT WITH 15" MOUNTING ARM BRACKET -WET RATED

RLM CLASSICS - WAREHOUSE SHADE DAMP/WET LOCATION WS1210GV-15L-35K-EX-TF2-MOUNTING ARM PA15 MATTE BLACK STANDARD COLOR OUTSIDE(WHITE INSIDE) WITH 4"

FROSTED MINI WIREGUARD CEILING FAN - 56" MODERNFORMS CERVANTES FAN, 4 BLADE INDOOR/ OUTDOOR SMART LED CEILING FAN WITH REMOTE, MATTE BLACK

BARNWOOD, 3500K

EXIT SIGNS (NOT SHOWN) ARE ASSUMED EXISTING AT EACH OF THE 8 DOORS AND ARE TO REMAIN AND/OR BE REHUNG AT EACH EXIT/ ENTRY DOOR IF IMPACTED BY CONSTRUCTION

CEILING PLAN KEYNOTES (#)

- 1 AREA OF NEW FRAMING AND ROOF AT REMOVED RAFTER AND ROOFING AT THE EXISTING BUILDING. REFER TO STRUCTURAL FOR NEW KNEE WALL ROOF SUPPORT AT EXISTING EXTERIOR WALL. CLAD WITH FIBER CEMENT BOARD &
- BATTEN SIDING, PAINTED. 2 TRIM EXISTING SOFFIT AND NEW SUPPORT BEAM WITH NEW PVC UNVENTED SOFFIT AND TRIM WITH BOARD AND BATTEN AT SIDE FOR NEW FINISHED LOOK. OVERFRAME KNEE WALL TO SUPPORT NEW ROOFLINE ABOVE. SEE STRUCTURAL

10

- 3 MOISTURE RESISTANT GWB (5/8") AT CEILING

PLAN & TRUE

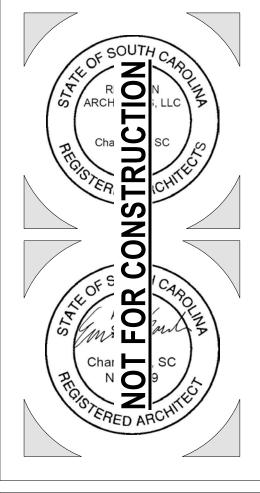
NORTH

- 4 OPEN TO STRUCTURE ABOVE
- 5 TAKE WALL TO UNDERSIDE OF ROOF DECK FULL HEIGHT 6 REPLACE EXISTING WALL PACK LIGHT WITH BRACKET LIGHT FIXTURE



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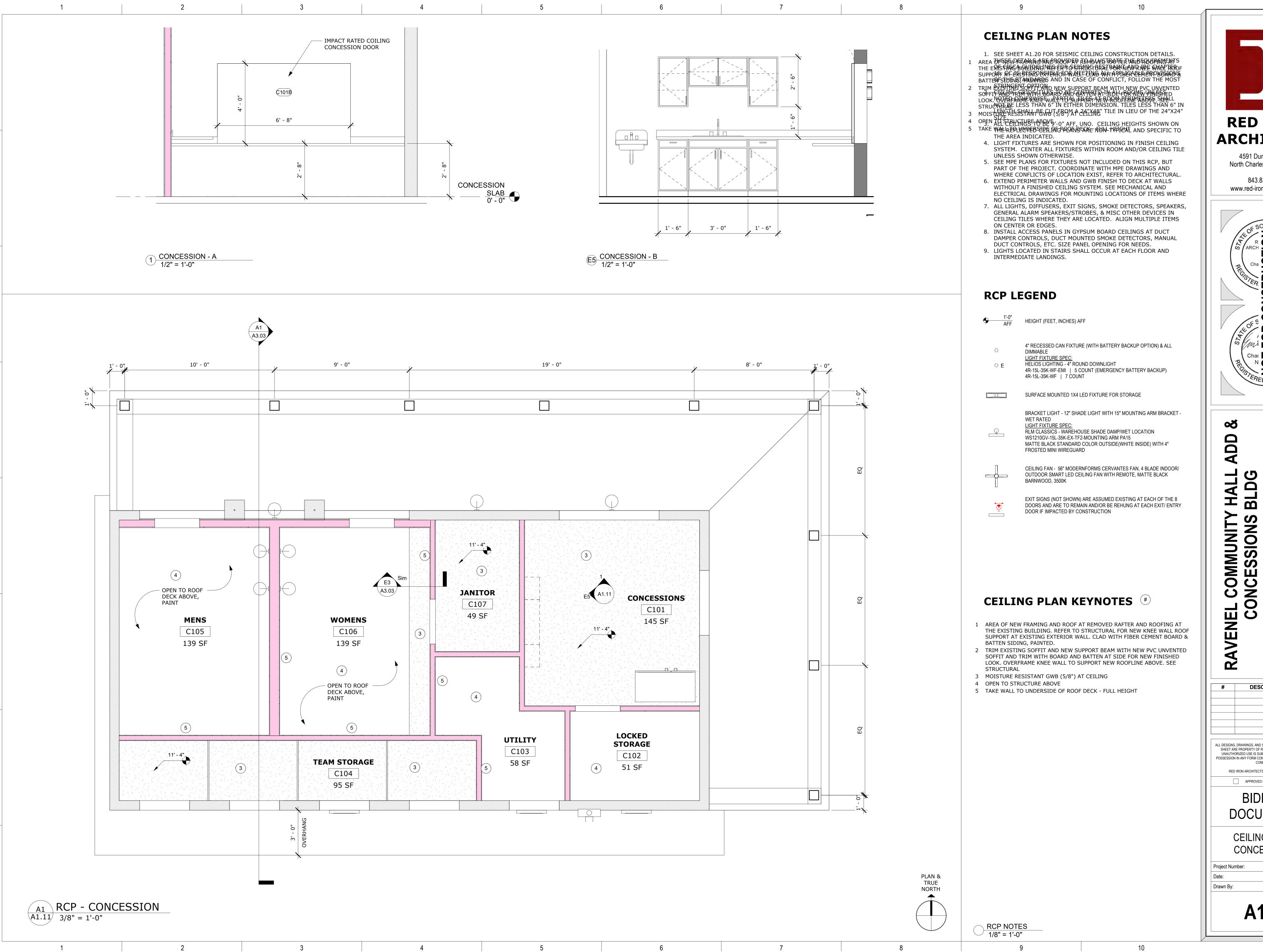
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CEILING PLAN - HALL

21081 Project Number: APRIL 19, 2023 Drawn By: Author



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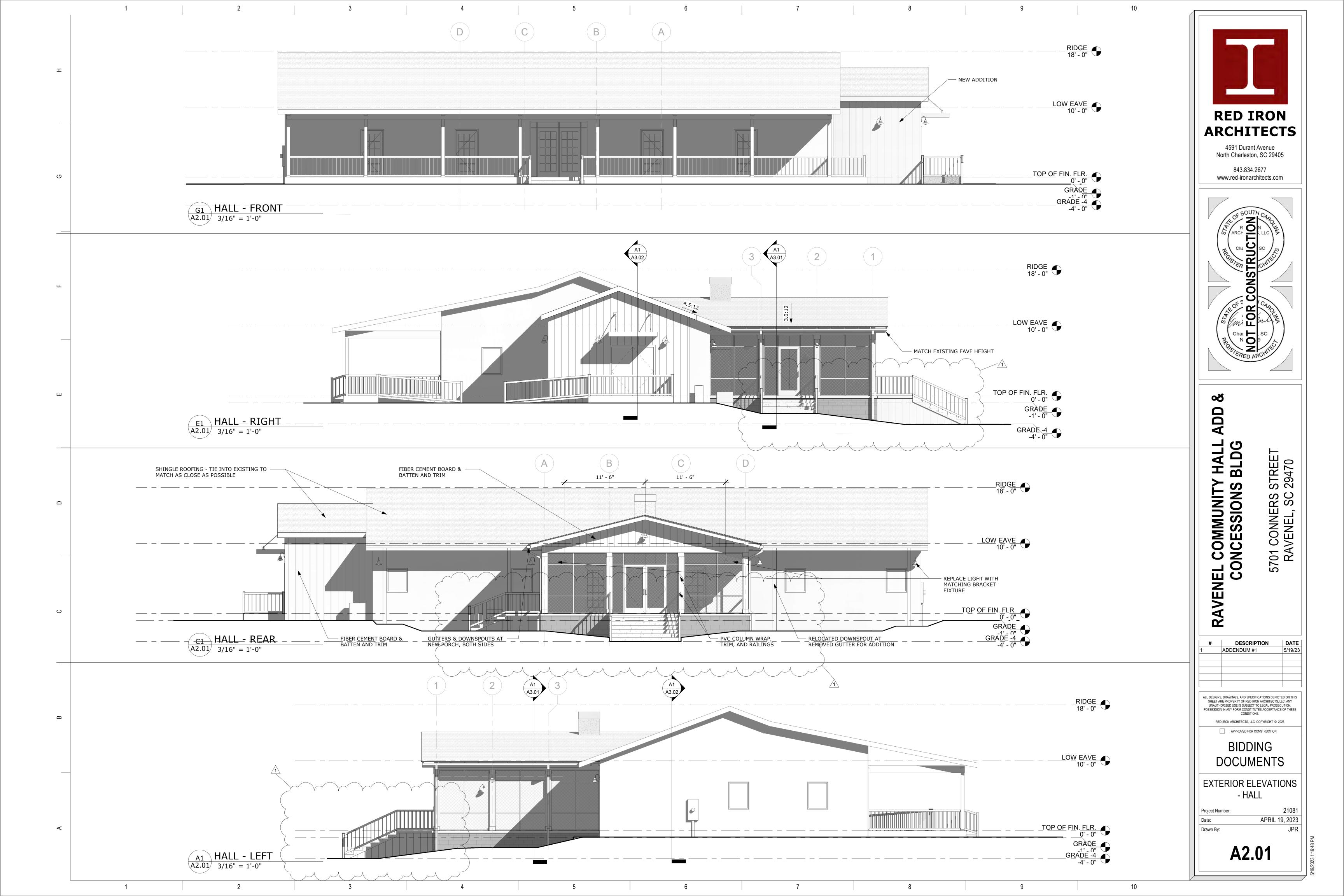
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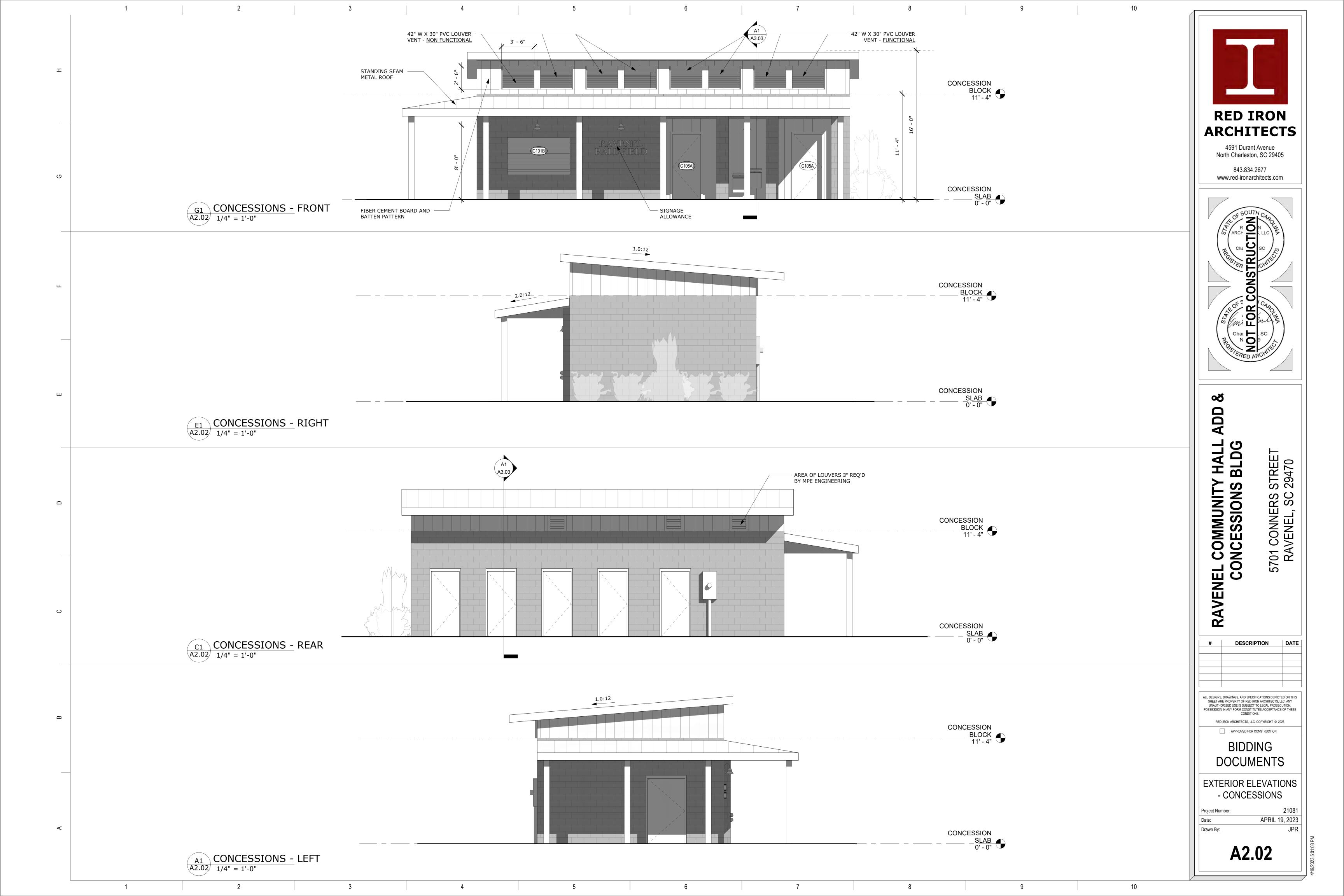
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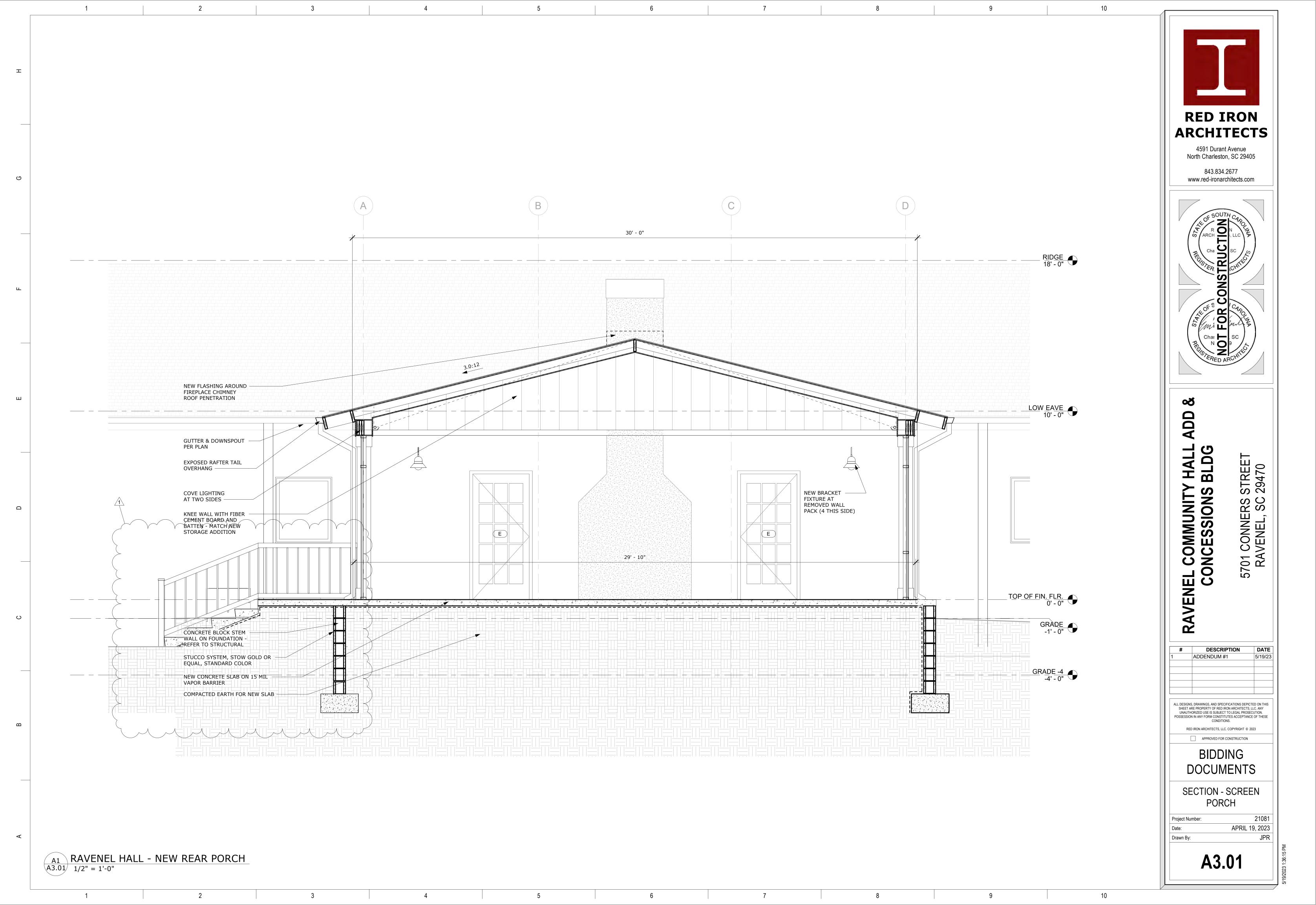
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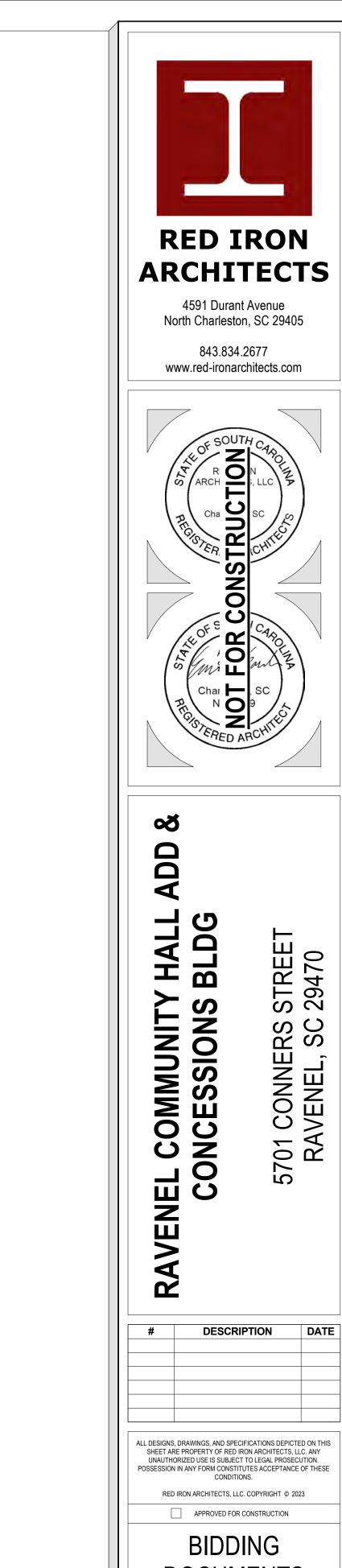
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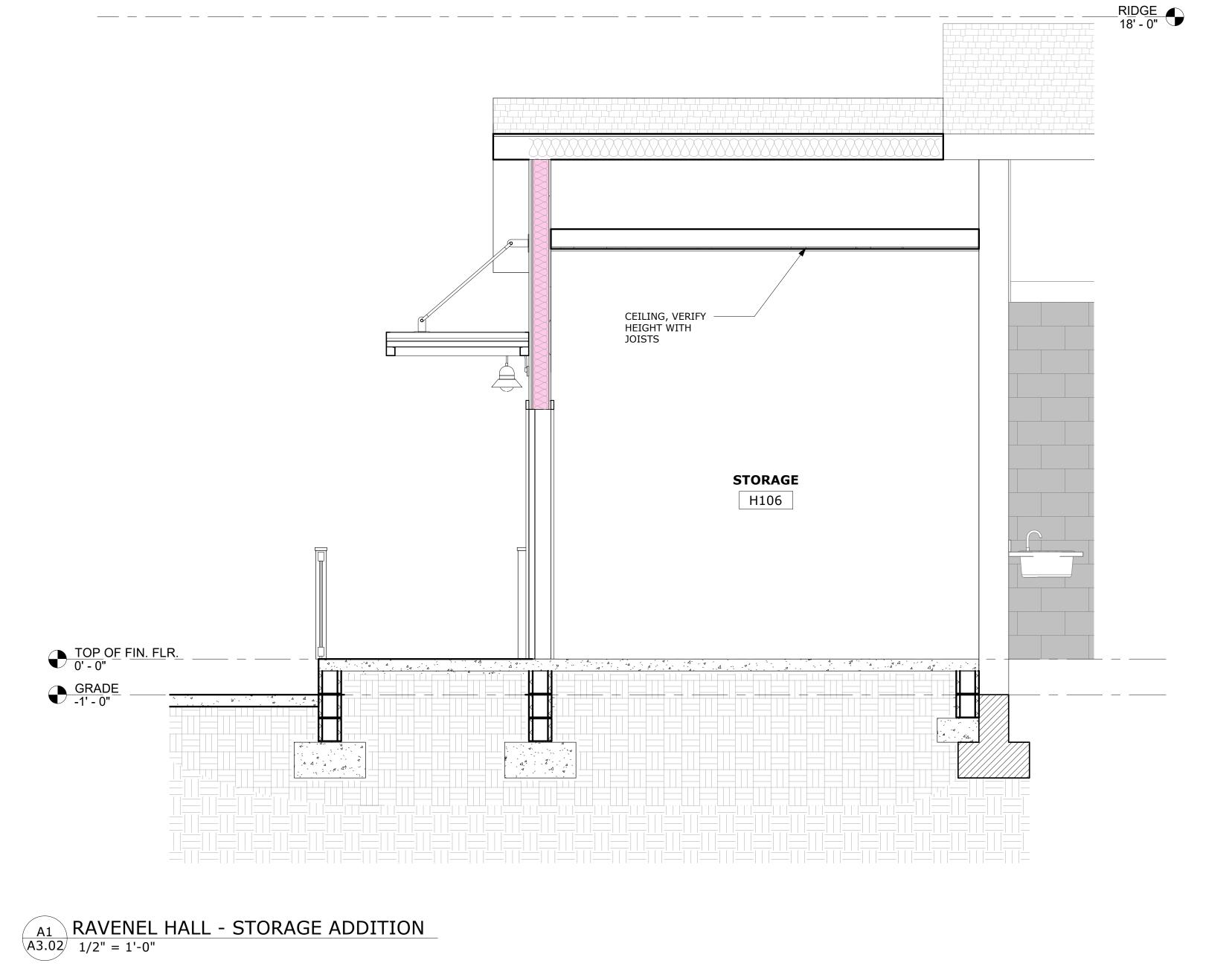
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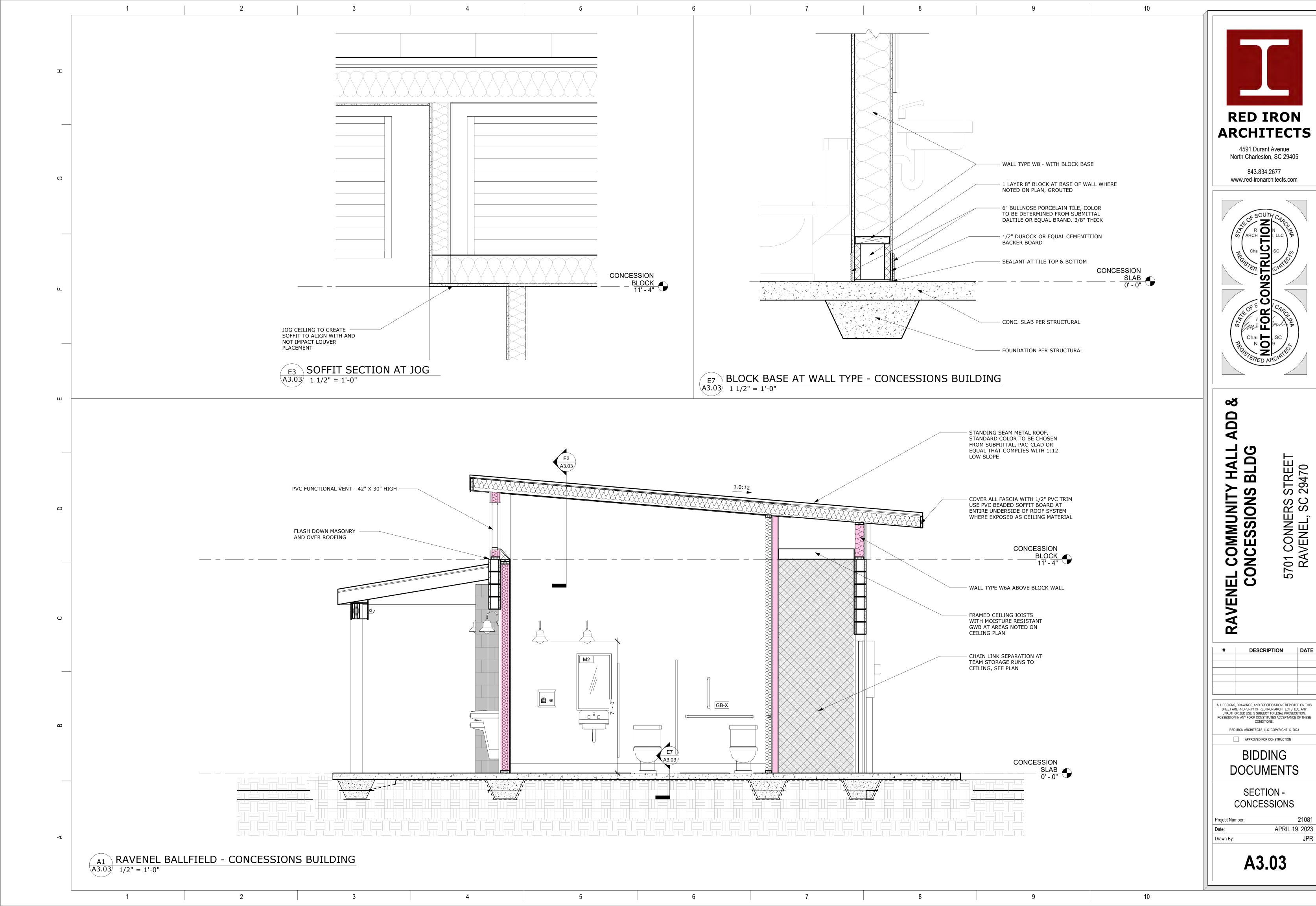
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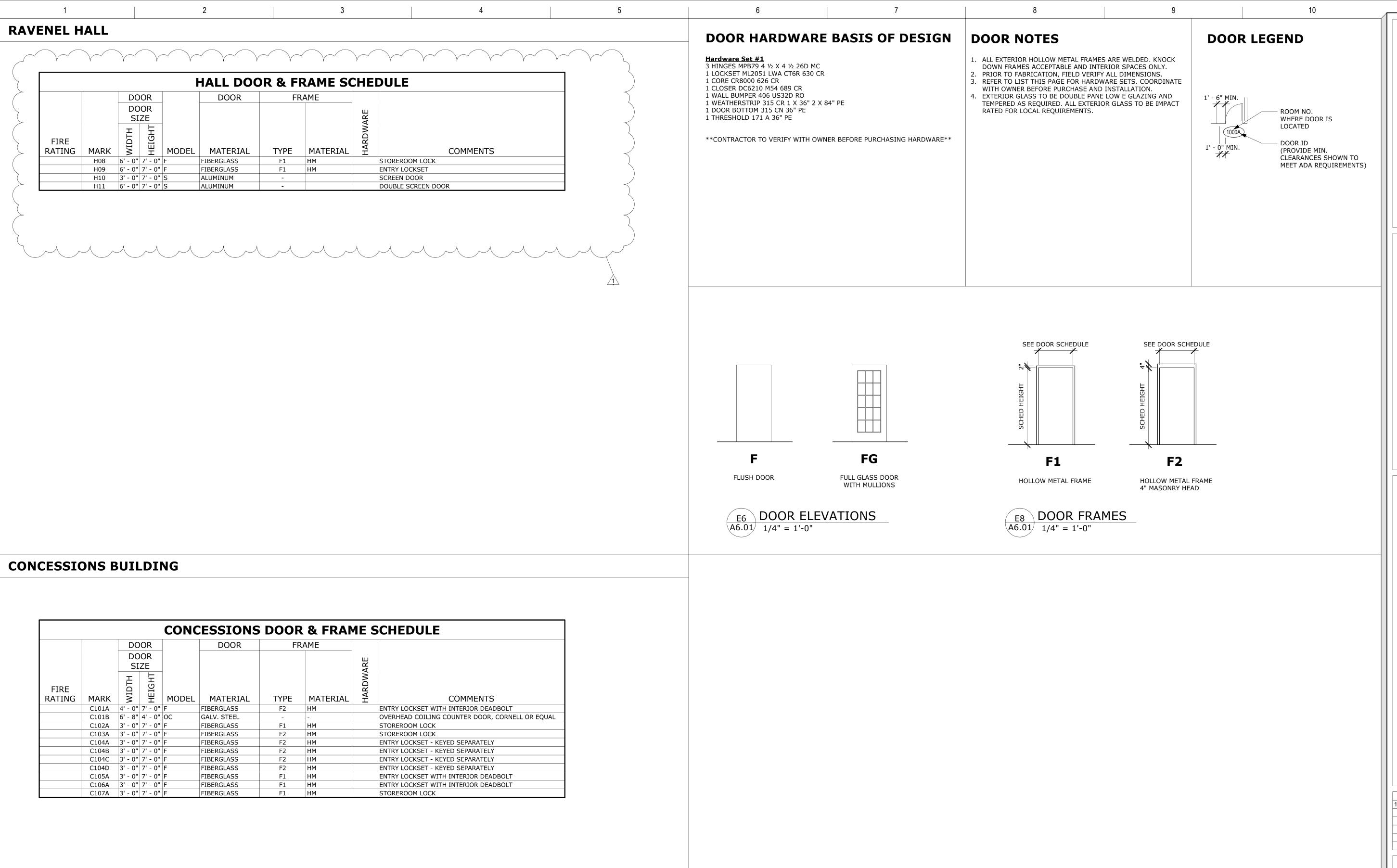
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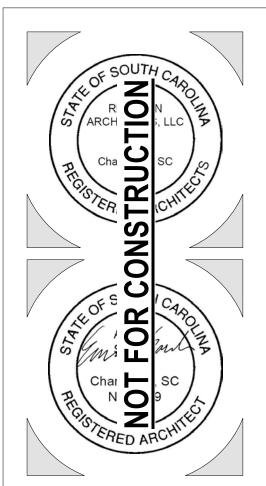
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